

Reference: #13S1210200

2 September 2015

JST Pty Ltd Level 2, 72 Macquarie Street PARRAMATTA NSW 2150

Attention: Mr. Tom Copping

Dear Tom

RE: BONDS SPINNING MILLS, PENDLE HILL – SUPPLEMENTARY TRAFFIC ASSESSMENT

1. Introduction

GTA Consultants had previously prepared a traffic and transport report in April 2014 for redevelopment of the Bonds Spinning Mills in Pendle Hill to incorporate approximately 1,600 new dwellings and 6,000m² of retail, including an approximately 4,000m² supermarket.

Subsequently, the Holroyd City Council has requested additional information on 20 June 2015. In July 2015, a revised masterplan was prepared by PTW Architects to incorporate approximately 1,700 new dwellings and 6,000m² retail area. Carparking is to be provided with a basement carpark.

As per the Council's letter dated 20 June 2014, the following items are addressed in the chapters specified below.

Issues Raised by Council	Addressed in Chapter(s)					
Item 1. The 40:60 in/out split for residential during the PM Peak shall be further justified. Alternatively the more traditional 70:30 split can be applied.	Section 5					
Item 2. The retail traffic distribution split between Gilba Road and Magowar Road should be reviewed considering Gilba Road is the higher order road and carries a more traffic than Magowar Road. It is more likely that the split is reversed (i.e. Gilba Road - 30% and Magowar Road - 5%).	Section 5					
Item 3. The I% per annum linear growth rate applied to background traffic is considered too low. The assessment of the future scenario shall consider the proposed intensification of the Pendle Hill Town Centre based on the recently updated Holroyd DCP and LEP and previous studies undertaken.	Section 5					
Item 4. The intersection modelling shall be extended to include the intersection of:						
i. Great Western Highway with Jones Street						
ii. Cumberland Highway with Dunmore Street						
iii. Cumberland Highway with Smith Street	Section 2 & 5					
Gilba Road with Pendle Way						
v. Wentworth Avenue with Goodall Street (if required by Parramatta City Council)						
vi. others as required by the Roads and Maritime Services (RMS).						

Table 1: Council's Comments

melbourne sydney

25YEARS

brisbane canberra adelaide gold coast townsville perth

Level 6, 15 Help Street CHATSWOOD NSW 2067 PO Box 5254 WEST CHATSWOOD NSW 1515 t// +612 8448 1800



Issues Raised by Council	Addressed in Chapter(s)
It is acknowledged that traffic generated by the proposed rezoning is unlikely to have an impact on the overall performance of these intersections however, minor traffic volume increases can make a difference to low priority movements (e.g. right turns, side roads) particularly at intersections which are saturated. In addition, the RMS will not increase green times on the side roads (i.e. Dunmore Street and Jones Street) as their aim is to maintain traffic flow along the highway. Therefore, any increase in traffic will likely increase delay queue lengths on the side road which will need to be addressed.	
Item 5. The Sidra modelling output summary shall include DOS and Queue Lengths.	Tables 2, 5 & 6
Item 6. The proposed access intersection treatments on Jones Street and Dunmore Street shall be detailed and assessed in regard to traffic (traffic modelling), road safety, loss of on-street parking and pedestrians.	Conceptual layouts are shown in Attachment 4
Item 7. An assessment of the Environmental Capacities of local and collector roads shall be undertaken to determine the impact on road safety and residential amenity. Based on the traffic generation analysis, Jones Street, Dunmore Street and possibly Smith Street will be above the limits. Measures to manage speed and maintain residential amenity shall be provided.	Section 5 Table 7
Item 8. The rezoning includes new access intersections and modification to existing intersections which will result in loss of on-street parking. Any loss of on-street parking will need to be detailed and addressed.	It should be noted that a significant number of new on street parking spaces will be created within the new internal roads, which will more than compensate for loss of parking at the site entries. Conceptual layouts are shown in Attachment 4
Item 9. Any changes to traffic signs (including parking restrictions), line marking and traffic devices are subject to the approval of the Holroyd Traffic Committee. Concept designs shall be prepared in accordance with Australian Standards, Austroads and RMS Supplements for reporting to the Holroyd Traffic Committee when issues have been resolved and further details have been provided.	Conceptual layouts are shown in Attachment 4
Item 10. The rezoning is a traffic generating development under SEPP (Infrastructure) 2007 and includes modification to signalised intersections which are subject to RMS approval. Therefore the proposal shall be referred to the RMS as soon as possible.	The proposal will be referred to RMS during agency consultation stage.
Item 11. Off-street parking including visitor parking shall be in accordance with Council's DCP.	Section 4

As this report provides supplementary traffic assessment to the submitted traffic report, this should be read in conjunction with GTA's Traffic and Transport Report¹ dated 30/04/14 (refer to Attachment 1).

2. Existing Conditions

We have undertaken additional intersection counts and queuing surveys at the following intersections to address Council's issue (refer to Item 4):

- Great Western Highway Jones Street
- Great Western Highway Pendle Way
- Cumberland Highway Dunmore Street
- Cumberland Highway Smith Street

¹ Bonds Spinning Mills Pendle Hill, Traffic and Transport Report, GTA Consultants, 30/04/14



- Gilba Road Pendle Way
- Wentworth Avenue Goodall Street.

The surveys were conducted on a Saturday 14 February 2015 between 11AM and 1PM and on Thursday 19 February 2015 between 4PM and 6PM. These periods coincide with the previous analyses submitted to council.

The intersection turning movement diagrams for Thursday PM and Saturday midday peak hours including the six intersections surveyed for the original traffic report are included in Attachment 2A.

Table 2 presents a summary of the existing operation of the study intersections with full results provided in Attachment 3A.

Intersection	Controls	Peak	Degree of Saturation (DOS)	Average Delay (sec)	95 th Percentile Queuing	Level of Service (LOS)
Jones Street/	Driesity	PM	0.14	8	7	А
Rogers Street	Phonty	Sat	0.11	7	4	А
Jones Street/	Driesity	PM	0.16	8	6	А
Oatlands Street	Phonty	Sat	0.11	7	4	А
Jones Street/	Poundabout	PM	0.32	11	14	А
Smith Street	Roundabout	Sat	0.21	10	8	А
Jones Street/	Poundabout	PM	0.63	15	43	А
Dunmore Street	Roundabout	Sat	0.39	12	18	А
Goodall Street/	Signals	PM	0.76	18	79	В
Dunmore Street	Signais	Sat	0.48	16	44	В
Pendle Way/	Signals	PM	0.65	18	73	В
Dunmore Street		Sat	0.65	15	41	В
Great Western	Signals	PM	0.62	20	157	В
Highway/ Jones Street	Signais	Sat	0.59	23	102	В
Cumberland Highway/	Signals	PM	0.83	38	276	С
Dunmore Street	Signais	SAT	0.86	38	183	С
Cumberland Highway/	Signals	PM	0.86	16	123	В
Smith Street	Signais	SAT	0.64	17	127	В
Pendle Way/	Driority	PM	0.85	41	43	С
Gilba Road	Phonty	SAT	0.86	39	46	С
Goodall Street/	Signals	PM	0.73	26	112	В
Wentworth Avenue	Signais	SAT	0.86	27	104	В
Cumberland Highway/	Signals	PM	0.93	43	191	D
Pendle Way	SIGLIAIS	SAT	0.99	42	134	С

 Table 2:
 Existing – Sidra Results

Table 2 indicates that all intersections currently operate satisfactorily with Level of Service (LoS) C or better, except Cumberland Highway/Pendle Way intersection.

Cumberland Highway/Pendle Way intersection operates near capacity (i.e. LoS D) during the Thursday PM peak period. It is expected that the majority of intersections along the arterial road would operate at or near capacity during the commuter peak periods. Hence,



intersections operating at Level of Service D during peak periods are generally considered acceptable.

3. Development Proposal

Following the submission of the Concept Mater Plan in 2014, the proposed development mix has been revised.

Table 3 presents the indicative development mix as provided by the Concept Masterplan as submitted to Council (adopted in the original GTA traffic report) against the current building envelopes as amended.

Table 3: Prop	osed Development Mix
---------------	----------------------

llee	Units/GFA				
Use	Previous Proposal	Current Proposal			
Residential					
-Dwellings	1,600 units*	1,700 units*			
Retail					
-Supermarket	4,000m ²	3,141m ²			
-Specialty retail	2,000m ²	2,823m ²			

NOTE: * - Unit numbers are indicative and subject to further refinement and unit mix employed during detailed planning phase.

Consistent with the traffic report¹, the following vehicle accesses are proposed:

- Dunmore Street-Access Road roundabout
- Jones Street-Access Road T-intersection.

The indicative concept layout for the access road intersections at Dunmore Street and Jones Street are included in Attachment 4. However, this is an indicative conceptual layout and further details will be confirmed during the DA stage.

4. Car Parking

The Concept Master Plan envisages that the following general on-site car parking provision rates:

- Residential at 1 space / dwelling = 1,700 residential parking spaces
- Visitor parking at 1 space / 5 dwellings = 340 residential visitor parking spaces
- Retail at 1 space / 44m2 GLFA = 140 retail parking spaces.

The above parking rates are consistent with the traffic report¹.

The envisaged provision of 2,180 car parking spaces accords with the minimum City of Holroyd DCP 2013 car parking requirements. Under the revised masterplan, the car parking spaces will be located within a basement. A significant additional on-street parking spaces will be provided within the new road reserves within the development.

Notwithstanding the above, the exact on-site parking provision will need to be defined once further details regarding the type of retail, commercial and residential land uses are proposed on the site, which will be in accordance with Council's DCP.



5. Traffic Impact Assessment

Traffic Generation

The traffic generation rates applied for this assessment are consistent with the original traffic assessment1. Table 4 presents the estimates of external peak hour traffic volumes resulting from the current proposal.

llee		Generation Rate			Movements/hr.			
Use	UTIIIS / GFA	AM	PM	Sat	AM	PM	Sat	
Residential	Residential							
Dwellings	1,700	0.32 per dwelling	0.18 per dwelling	0.23 per dwelling	544	306	391	
Retail								
Supermarket	3,141m ²	58 per 1,000m²	116 per 1,000m ²	110 per 1,000m ²	182	364	345	
Specialty retail	2,823m ²	17.5 per 1,000m ²	35 per 1,000m²	80 per 1,000m ²	50	99	226	
Total		776	769	962				

Table 4: Traffic Generation Estimates (External Trips)

Table 4 indicates that the site could potentially generate approximately 780 external vehicle movements in the AM peak hour, 770 external vehicle movements in the PM peak hour and 960 external vehicle movements in the Saturday midday peak hour.

Despite the increase in unit numbers from the previous proposal, the above external movements for the proposed development are lower than the previously assessed development of approximately 800 external vehicle movements in the AM peak hour, 840 external vehicle movements in the PM peak hour and 1,000 external vehicle movements in the Saturday midday peak hour.

For this supplementary assessment, we have focused on traffic impact assessment during the Thursday PM and Saturday midday periods, as requested by Council in previous submission in order to ensure that the impact of the development generated traffic coincided with the busiest times in the town centre, which is in close vicinity of the site.

Trip Distribution

As requested by the Council (refer to Item 1), the following directional split of traffic (i.e. the ratio between the inbound and outbound traffic movements) has been adopted:

- Residential: 20% inbound/ 80% outbound during the Thursday AM peak, and 70% inbound/ 30% outbound during the Thursday PM peak.
- Retail: 70% inbound/ 30% outbound during the Thursday AM peak, and 60% inbound/ 40% outbound during the Thursday PM peak.
- During the Saturday midday peak, the directional split of both retail and residential traffic has been assumed as 50% outbound/ 50% inbound.

In addition, the PM peak hour residential directional split has been revised to 70% inbound/ 30% outbound.

In addition to above, the anticipated retail traffic distribution was revised as suggested by the Council (refer to Item 2). This is presented in Figure 1.





Figure 1: Traffic Distribution - Retail Component

The revised retail trip distribution assumed that 30% of retail trips would be to/from Gilba Road and 5% would travel to/from Magowar Road.

For the development traffic generated by the retail component, it was assumed that these trips would be generally contained within the boundary of Great Western Highway and Cumberland Highway. It is anticipated that the majority of customers of the proposed local retail facilities would be the local residents of Pendle Hill.

Figure 2 shows the location of other nearby retail centres. It can be seen that would be unlikely for the potential customers to travel along Great Western Highway and/or Cumberland Highway since there are already other regional retail centres nearby.



Figure 2: Location of Other Retail Centres



Future Impacts

GTA has previously applied 1% background traffic growth per annum on the existing road network. However, following the Council's comment (refer to Item 3), we have reviewed the Bureau of Transport Statistics' (BTS) data.

BTS maintains the Strategic Travel Model (STM) for projecting travel patterns in New South Wales. A broad range of assumptions are included in the model and as such, results should only be used as an indicative measure.

A review of Strategic Travel Model data at Pendle Hill suggested that the following traffic growth would be appropriate:

- 1.5% growth per annum along all local streets
- 1% growth per annum along the main highways (i.e. Great Western Highway and Cumberland Highway).

The above traffic growth percentages are applied to the existing intersection flows up to the future year, 2027. Intersection turning movement diagrams for 2027 Thursday PM and Saturday midday peak hours are included in Attachment 2B.

Intersection turning movement diagrams for 2027 with development generated traffic are also included in Attachment 2C.



Intersection Assessment

Following the distribution and assignment of development traffic and application of growth to background traffic, the study intersections were reanalysed using SIDRA INTERSECTION.

Table 5 summarises the SIDRA results of the Year 2027 'without development' and 'with development' scenarios in terms of average delay and level of service.

The full results from the SIDRA analysis are presented in Attachment 3B and 3C for the Year 2027 'without development' and 'with development' scenarios, respectively.

Intersection assessment results shown in Table 5 indicates that all sign controlled and roundabout intersections would operate satisfactorily with LoS C or better for both peak periods, except for the Pendle Way/Gilba Road intersection.

Pendle Way/Gilba Road intersection would operate at LoS F in 2027 under post development conditions. Hence, mitigation measures would be necessary to improve performance of this intersection. Notwithstanding the above, the operation of this intersection is largely affected by the activities along Pendle Way, north of the intersection such as delays due to parking manoeuvres, pedestrian crossings generated by the retail strip etc. The vehicle queues generated by these activities downstream of the Pendle Way/Gilba Road intersection, obstruct vehicles from passing through this intersection.

It is noted that in order to replicate the observed queue length on north and south approaches of Pendle Way using SIDRA INTERSECTION, the capacity of these approaches have been reduced by 40-45% for both existing and 2027 scenarios. This has significant impact on the 2027 post development SIDRA results as additional development traffic was added on to the network.

Therefore, mitigation measures are likely to require the consideration of the effect of retail activities in vicinity of the Pendle Hill station, which is located downstream from Pendle Way/Gilba Road intersection rather than increasing the capacity of this intersection itself.

Cumberland Highway intersections with Pendle Way and Dunmore Street would operate near capacity (i.e. LoS D) in 2027 with or without the development traffic added to the road network. As mentioned in Chapter 2, it is expected that the majority of intersections along the arterial road would operate at or near capacity during the commuter peak periods. Hence, intersections operating at Level of Service D during peak periods are generally considered acceptable.

It is noted that Goodall Street/Wentworth Avenue intersection would operate near capacity (i.e. LoS D) in 2027 with the development traffic added to the road network. The existing traffic signal timings have been adopted at this signalised intersection. Further assessment of this intersection indicated that the level of service could be improved to "B", if traffic signal timing is revised to reflect the additional traffic on Goodall Street.

As requested by the Council (refer to Item 4), all traffic signal timings have been fixed based on the existing Intersection Diagnostic Monitor (IDM) data obtained from RMS.



		Peak	'Without Development'				'With Development'			
Intersection	Controls		Degree of Saturation (DOS)	Average Delay (sec)	95 th Percentile Queuing	Level of Service (LOS)	Degree of Saturation (DOS)	Average Delay (sec)	95 th Percentile Queuing	Level of Service (LOS)
Jones Street/	Driority	PM	0.17	9	8	А	0.20	9	10	А
Rogers Street	Phonty	Sat	0.13	8	5	А	0.17	8	7	А
Jones Street/	Driority	PM	0.19	9	8	А	0.28	11	18	А
Oatlands Street	Phonty	Sat	0.13	8	5	А	0.28	12	16	А
Jones Street/	Roundabout	PM	0.39	11	19	А	0.53	15	30	В
Smith Street	Roundabout	Sat	0.26	10	11	А	0.49	12	27	А
Jones Street/	Doundahout	PM	0.80	21	86	В	0.92	31	165	С
Dunmore Street	Roundabout	Sat	0.48	13	24	А	0.61	14	40	А
Goodall Street/	Signals	PM	0.80	20	101	В	0.87	23	117	В
Dunmore Street		Sat	0.59	17	44	В	0.64	19	90	В
Pendle Way/	Signals	PM	0.78	20	101	В	0.82	22	128	В
Dunmore Street		Sat	0.78	16	54	В	0.81	18	86	В
Great Western	Signals	PM	0.77	22	188	В	0.87	23	188	В
Highway/ Jones Street		Sat	0.75	24	118	В	1.13	30	122	С
Cumberland Highway/	Signals	PM	1.01	44	337	D	1.03	46	337	D
Dunmore Street		SAT	1.07	49	305	D	1.10	45	323	D
Cumberland Highway/	Cierrola	PM	1.05	21	205	В	1.11	23	200	В
Smith Street	signais	SAT	0.80	18	150	В	1.14	23	159	В
Pendle Way/	Driesity	PM	0.59	24	30	В	1.61	612	438	F
Gilba Road	Phonty	SAT	0.69	27	37	В	2.06	1015	632	F
Goodall Street/	Cierrola	PM	0.87	29	139	С	0.97	30	139	С
Wentworth Avenue	signais	SAT	0.92	27	102	В	1.13	47	233	D
Cumberland Highway/	Cianala	PM	1.10	49	248	D	1.10	50	253	D
Pendle Way	signais	SAT	1.00	44	155	D	1.00	45	157	D

Table 5: Year 2027 'Without Development' and 'With Development' scenarios - SIDRA Results



Access Road Intersections

The indicative concept layout for the access road intersections at Dunmore Street and Jones Street are included in Attachment 4. The following form of access is proposed:

- Dunmore Street-Access Road roundabout
- Jones Street-Access Road T-intersection.

The results of the intersection performance are presented in Table 6.

Access Location	Controls	Peak	Degree of Saturation (DOS)	Average Delay (sec)	95 th Percentile Queuing	Level of Service (LOS)
Dunmara Streat	Roundabout	PM	0.86	22	132	В
Dunmore Street		Sat	0.72	20	69	В
lonos Stroot	Priority	PM	0.30	19	15	В
Jones Slieer		Sat	0.45	18	17	В

Table 6: Year 2027 Access Road – SIDRA Results

As demonstrated above, both Dunmore Street and Jones Street access road intersections would operate at a good level of service (i.e. LoS B) with acceptable delays for both peak periods under the 2027 post development scenarios.

Environmental Capacity Assessment

A copy of the Environmental capacity performance standards on residential streets table from RMS *Guide to Traffic Generating Development* is produced below.

 Table 4.6

 Environmental capacity performance standards on residential streets

Road class	Road type	Maximum Speed (km/hr)	Maximum peak hour volume (veh/hr)
	Access way	25	100
Local	Street	40	200 environmental goal
		40	300 maximum
Collector	Streat	50	300 environmental goal
Conector	Sileet	50	500 maximum

Note: Maximum speed relates to the appropriate design maximum speeds in new residential developments. In existing areas maximum speed relates to 85th percentile speed.

Table 7 presents the two-way midblock flows on the collector and local residential streets in vicinity of the site.



Road Sections	Existing Flows		2027 'Without Development' Flows		2027 'With Development' Flows	
	PM	Sat	PM	Sat	PM	Sat
Collector Streets:						
Dunmore St, east of Jones St	842	653	1006	780	1144	975
Dunmore St, east of Goodall St	1124	899	1343	1074	1589	1482
Jones St, north of Dunmore St	195	131	232	157	232	157
Jones St, south of Dunmore St	495	339	592	405	684	541
Jones St, south of Oatlands St	513	351	613	419	974	945
Pendle Way, north of Dunmore St	1008	940	1205	1123	1344	1356
Goodall St, north of Dunmore St	1076	732	1286	875	1356	991
Smith St, east of Jones St	550	344	657	411	765	567
Local Streets:						
Rogers St, east of Jones St	33	35	39	42	63	81
Oatlands St, east of Jones St	59	45	71	54	94	93

Table 7: Peak Hour Two-Way Flows on Residential Streets

Table 7 indicates that except at Jones Street, north of Dunmore Street, all of the above collector type residential streets currently do not meet the environmental capacity threshold of 500 vehicles per hour (albeit that the above criteria are for purely residential roads rather than mixed use roads) with or without the development traffic added to the 2027 road network. The RMS Guide to Traffic Generating Development recognises that "the Environmental Capacity of a street can be increased through a reduction in speed" and that "traffic speed may be reduced by the introduction of traffic calming methods".

We consider that the following traffic calming devices could be considered but this would usually be the subject of further discussions at a more advanced stage of scheme development:

- Road humps to stop vehicles from speeding up but it may not be suitable along the bus routes.
- Speed cushions one to three devices could be installed, depending on the width of the road. Speed cushions are most accommodating for vehicles such as buses and emergency vehicles compared to the road humps.
- Speed tables similar to road humps but longer and with a flattened top, sometimes used to give pedestrians a level crossing between footways.
- Pedestrian refuges allow pedestrians to cross one stream of traffic at a time. It is designed to reduce the speed by narrowing the road, however the road needs to be wide enough to allow for a suitable refuge and the safe passage of vehicles and cycles.

The above table also indicates that all local streets such as Rogers Street and Oatlands Street are expected to meet the environmental capacity threshold.

6. Conclusion

This supplementary traffic assessment report has addressed the issues raised by the Council.



In conclusion, the proposed redevelopment would be able to proceed without having a significant adverse impact on the overall performance of the road network in the vicinity of the site, albeit that some local intersection improvements may be required at Pendle Way/Gilba Road intersection which is likely to require the consideration of the effect of retail activities downstream of this intersection rather than increasing the capacity of itself and modification of traffic signal timing at Goodall Street/ Wentworth Avenue intersection to reflect the additional traffic on Goodall Street.

Furthermore, traffic calming measures may be required to manage speed and to minimise any environmental effects on residential amenity. Details of intersection upgrades and traffic calming options would be addressed and approved by Council during the detail design stage.

Naturally, should you have any questions or require any further information, please do not hesitate to contact me in our Sydney office on (02) 8448 1800.

Yours sincerely

GTA CONSULTANTS

Ken Hollyoak Director (NSW)

encl.



Attachment 1

Traffic and Transport Report, GTA Consultants (30 April 2014)





Bonds Spinning Mills

Pendle Hill

Traffic and Transport Report

Issue: E 30/04/14

Client: Dyldam Development Pty Ltd Reference: 13S1210100 GTA Consultants Office: NSW

Issue	Date	Description	Prepared By	Checked By	Approved By	Signed
А	13/12/12	Final	Sarah Court	Ken Hollyoak	Ken Hollyoak	Ken Hollyoak
В	22/05/13	Revised Final	Jason Rudd	Ken Hollyoak	Ken Hollyoak	Ken Hollyoak
С	05/06/13	Revised Final	Jason Rudd	Ken Hollyoak	Jason Rudd	Jason Rudd
D	11/04/14	Incorporating Council and RMS comments	Ashish Modessa	Wayne Johnson	Ken Hollyoak	Ken Hollyoak
E	30/04/14	Amended Car Parking Provisions	Ashish Modessa	Wayne Johnson	Ken Hollyoak	KI Huy-L

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1. Introduction

1.1 Background

Dyldam Development Pty Ltd is currently investigating the redevelopment of the Bonds Spinning Mills in Pendle Hill to incorporate approximately 1,600 new dwellings and 6,000m² of retail, including an approximately 4,000m² supermarket. The final dwelling numbers/mix as well as the retail floor areas would be confirmed at a later stage.

The site is located close to public transport and is within easy walking and/or cycling distance of key destinations such as Pendle Hill Station, Westmead Hospital precinct and the Parramatta CBD. Such a location provides an opportunity to create a sustainable urban neighbourhood which promotes and encourages sustainable transport, and provides a catalyst for urban renewal and revitalisation in and around the Pendle Hill area.

GTA Consultants was commissioned by CBRE to undertake a transport impact assessment for the proposed development. This report has also taken into account comments from the City of Holroyd Council meeting in October 2013.

1.2 Purpose of this Report

This report sets out an assessment of the anticipated transport implications of the proposed development, including consideration of the following:

- i existing traffic and parking conditions surrounding the site;
- ii existing pedestrian, bicycle and public transport facilities and accessibility;
- iii pedestrian, bicycle & public transport needs, in particular with regard to site layout;
- iv service vehicle requirements;
- v the traffic generating characteristics of the proposed development;
- vi suitability of the proposed access arrangements for the site; and
- vii the transport impact of the development proposal on the surrounding road network.

1.3 References

In preparing this report, reference has been made to the following:

- an inspection of the site and its surrounds;
- City of Holroyd Development Control Plan (DCP) and Local Environment Plan (LEP);
- Australian Standard/ New Zealand Standard, Parking Facilities, Part 1: Off-Street Car Parking AS/NZS 2890.1:2004;
- Australian Standard, Parking Facilities, Part 2: Off-Street Commercial Vehicle Facilities AS 2890.2:2002;
- Australian Standard / New Zealand Standard, Parking Facilities, Part 6: Off-Street Parking for People with Disabilities AS/NZS 2890.6:2009;
- City of Holroyd Ordinary Council Meeting 29 October 2013 at 6:30pm;
- traffic and car parking surveys undertaken by Colston Budd Hunt & Kafes as referenced in *Transport Report for Proposed Mixed Use Residential Development*, *Pendle Hill*, dated July 2012;
- traffic surveys undertaken by Skyhigh Traffic Data in March 2014;
- correspondence relating to proposed development configuration; and
- other documents and data as referenced in this report.



2. Existing Conditions

The subject site is located at 190-192 Dunmore Street, Pendle Hill. The site has frontage to Dunmore Street (~230m) and Jones Street (~320m), and comprises 8 hectares with approximately 47,000m² of existing industrial/warehouse development.

The site currently has a land use classification as Light Industrial and is occupied by the Pacific Brands industrial/ manufacturing facility, administration, storage and distribution operations. Surrounding land use is predominately residential and commercial/ retail, with the Pendle Hill Retirement Village located just west of the site.

Access to the site is currently provided from both Dunmore Street and Jones Street via combined entry/exit driveways.

The location of the subject site and its surrounding environs is shown in Figure 2.1.



Figure 2.1: Subject Site and Its Environs

(Source: Google Maps)

2.1 Road Network

The major road network in the vicinity of the site includes the Great Western Highway and the M4 Western Motorway running east-west to the south of the site and the Cumberland Highway running north-south to the east of the site. These arterial roads connect the site to major destinations including the Sydney CBD, Parramatta and Liverpool.



2.1.1 Adjoining Roads

Dunmore Street

Dunmore Street is a classified Regional Road and in the vicinity of the site is aligned in an eastwest direction. It is a two-way road configured with a 4-lane, comprising a traffic lane and a kerbside parking lane in each direction, 12m wide carriageway, set within an approximately 20m wide road reserve.

Kerbside parking is permitted in certain locations, including along the perimeter of the site. Dunmore Street is shown in Figure 2.2 and carries approximately 12,000 vehicles per day¹ (in the vicinity of the site).

Jones Street

Jones Street is a local road and in the vicinity of the site is aligned in a north-south direction. It is a two-way road configured with a 2-lane, 12m wide carriageway, set within an approximately 20m wide road reserve.

Kerbside parking is permitted, subject to time restrictions. Jones Street is shown in Figure 2.3 and carries approximately 5,000 vehicles per day² (in the vicinity of the site).

Figure 2.2: Dunmore Street



Figure 2.3: Jones Street



Pendle Way

Pendle Way is a local road and in the vicinity of the site is aligned in a north-south direction. It is a two-way road configured with a 2-lane, 12m wide carriageway, set within an approximately 20m wide road reserve.

Kerbside parking is permitted and is unrestricted. Pendle Way is shown in Figure 2.4 and carries approximately 11,000 vehicles per day³ (in the vicinity of the site).

Smith Street

Smith Street is a local road and in the vicinity of the site is aligned in an east-west direction. It is a two-way road configured with a 2-lane, 12m wide carriageway, set within an approximately 20m wide road reserve.

Kerbside parking is permitted and is unrestricted. Smith Street is shown in Figure 2.5 and carries approximately 5,500 vehicles per day⁴ (in the vicinity of the site).

¹ Based on tube count data collected between Jones Street and Goodall Street in March 2014.

² Based on tube count data collected between Jones Street and Goodall Street in March 2014.

³ Based on the peak hour traffic counts undertaken by Colston Budd Hunt in 2012 (factored for a Thursday PM peak hour) and assuming a peak-to-daily ratio of 10% for local roads.

Existing Conditions



Figure 2.4: Pendle Way



Figure 2.5: Smith Street



Rogers Street

Rogers Street is a local road and in the vicinity of the site is aligned in an east-west direction. It is a two-way road configured with a 2-lane, 10m wide carriageway, set within a 20m wide road reserve (approximate).

Kerbside parking is permitted and is unrestricted. Rogers Street is shown in Figure 2.1 and carries less than 400 vehicles per day⁵.

Oatlands Street

Oatlands Street is a local road and in the vicinity of the site is aligned in an east-west direction. It is a two-way road configured with a 2-lane, 12m wide carriageway, set within a 20m wide road reserve (approximate).

Kerbside parking is permitted and is unrestricted. Oatlands Street is shown in Figure 2.7 and carries less than 600 vehicles per day⁶.



Figure 2.7: Oatlands Street



4 Based on the peak hour traffic counts undertaken by Colston Budd Hunt in 2012 (factored for a Thursday PM peak hour) and assuming a peak-to-daily ratio of 10% for local roads.

⁵ Based on the peak hour traffic counts undertaken by Colston Budd Hunt in 2012 (factored for a Thursday PM peak hour) and assuming a peak-to-daily ratio of 10% for local roads.

⁶ Based on the peak hour traffic counts undertaken by GTA Consultants in 2012 (factored for a Thursday PM peak hour) and assuming a peak-to-daily ratio of 10% for local roads.



2.1.2 Surrounding Intersections

The following intersections currently exist in the vicinity of the site:

- Dunmore Street/ Jones Street (roundabout)
- Dunmore Street/ Goodall Street (signalised intersection)
- Dunmore Street/ Pendle Way (signalised intersection)
- Jones Street/ Smith Street (roundabout)
- Jones Street/ Rogers Street (unsignalised intersection)
- Jones Street/ Oatlands Street (roundabout).

2.2 Traffic Volumes

As part of a previous planning proposal submission for the subject site, Colston Budd Hunt and Kafes (CBHK) on behalf of Pacific Brands, undertook traffic movement counts on key roads/ intersections in the vicinity of the site during the AM and PM peak period.

GTA Consultants supplemented these counts by undertaking traffic movement counts at the intersection of Jones Street/ Oatlands Street on Tuesday 27 November 2012 during the AM and PM peak hours, determined to be 7.30am-8.30am and 5.00pm to 6.00pm, respectively.

Following comments from City of Holroyd Council and Roads and Maritime Services (RMS), the study intersections were surveyed on Saturday 8th March 2014 during the midday peak hour, determined to be 11:00am-12:00pm.

In addition, classified traffic volumes were collected on Dunmore Street and Jones Street for 7days starting 5th March 2014 to estimate the change in traffic on a Thursday in 2014 as compared to a Tuesday in 2012. In this way the existing Tuesday counts could be factored to replicate the Thursday conditions required by Council. As a result, all movements along Dunmore Street were increased by 2% and 5% during the weekday AM and PM peak hour periods respectively. Similarly, all movements along Jones Street were reduced by 6% and increased by 10% during the weekday AM and PM peak hour periods respectively.

The estimated Thursday AM and PM peak hour and surveyed Saturday midday traffic volumes are summarised in Figure 2.8, Figure 2.9 and Figure 2.10, respectively.

2.2.1 Historic / Potential Re-Use of Site

In addition to the surveyed intersections, Colston Budd also undertook traffic counts of the number of vehicles entering and exiting the site from Dunmore Street and Jones Street. These counts indicated that the existing facility on the site generated between 50-60 vehicles per hour two-way during the morning and afternoon peak. Historically, the site would have generated significantly more traffic than this. Indeed, the Colston Budd report noted the following:

"over recent years, the industrial and manufacturing operations on the site have been scaled down and as a consequence, a number of existing buildings within the site are either vacant or underutilised. At peak operation the site generated significant heavy vehicle movements, including articulated vehicles, and significant on-site employee parking. The site operated 24 hours per day seven days per week and had an employment work force of some 2,000 staff. The site currently has substantial fewer staff, with some 400 people employed on the site".



With approximately 47,000m² of existing industrial/warehouse development spread over 8 hectares, the existing site has significantly higher potential to generate traffic than it currently does. If the existing building footprint on-site was to be used to full capacity (without redevelopment) the site has the potential to generated 565 trips per hour in the peak periods. Given the nature of the use, a significant number of these would presumably be by trucks.

If redeveloped for an industrial or warehousing / distribution use, for which the current zoning allows, it is likely that a developer would increase the floor space ration (FSR) from the current provision, and as such the traffic generated could be much higher than 565 trips per hour.

Based on surrounding development patterns of 0.5:1 for R2 Low Density Residential and 1.2:1 for the R4 High Density Residential, the subject site, if developed for industrial use to an FSR of, say, 0.85:1, could potentially generate around 800 trips per hour.





















2.3 Intersection Operation

The operation of the key intersections within the study area have been assessed using SIDRA INTERSECTION⁷, a computer based modelling package which calculates intersection performance.

The commonly used measure of intersection performance, as defined by the RMS, is vehicle delay. SIDRA INTERSECTION determines the average delay that vehicles encounter and provides a measure of the level of service.

Table 2.1 shows the criteria that SIDRA INTERSECTION adopts in assessing the level of service.

			n de la constanción d
Level of Service (LOS)	Average Delay per vehicle (secs/veh)	Traffic Signals, Roundabout	Give Way & Stop Sign
А	Less than 14	Good operation	Good operation
В	15 to 28	Good with acceptable delays and spare capacity	Acceptable delays and spare capacity
С	29 to 42	Satisfactory	Satisfactory, but accident study required
D	43 to 56	Near capacity	Near capacity, accident study required
E	57 to 70	At capacity, at signals incidents will cause excessive delays	At capacity, requires other control mode
F	Greater than 70	Extra capacity required	Extreme delay, major treatment required
N/A	Intersection LOS and Major Road Approach LOS values are Not Applicable for two- way sign control since the average delay is not a good LOS measure due to zero delays associated with major road lanes.		

 Table 2.1:
 SIDRA INTERSECTION Level of Service Criteria

Table 2.2 presents a summary of the existing operation of the study intersections with full results provided in Appendix A of this report.

⁷ Program used under license from Akcelik & Associates Pty Ltd.



Intersection	Peak	Leg	Average Delay (sec)	Level of Service (LOS)
		South	1	NA
	AM	East	11	А
		North	0	NA
		All	1	NA
		South	2	NA
Jones Street/ Rogers Street	PM	East	10	А
		North	0	NA
		All	1	NA
		South	1	NA
	Sat	East	9	А
		North	0	NA
		All	1	NA
		South	1	NA
		East	9	А
	Alvi	North	0	NA
		All	1	NA
		South	2	NA
Jones Street/ Oatlands	PM	East	10	А
Street		North	0	NA
		All	2	NA
	Sat	South	2	NA
		East	8	А
		North	0	NA
		All	1	NA
	AM	South	7	А
		East	7	А
		North	9	А
		West	8	А
		All	8	А
	PM	South	8	А
		East	7	А
Jones Street/ Smith Street		North	7	А
		West	7	А
		All	7	А
	Sat	South	7	А
		East	7	A
		North	7	А
		West	7	A
		All	7	A

Table 2.2: Existing – Sidra Results



Existing Conditions

Intersection	Peak	Leg	Average Delay (sec)	Level of Service (LOS)
		South	10	А
	АМ	East	8	А
		North	15	В
		West	7	А
		All	8	А
		South	12	А
		East	9	А
Jones Street/ Dunmore	PM	North	11	А
Succi		West	7	А
		All	9	A
		South	9	A
		East	7	A
	Sat	North	10	A
		West	7	A
		All	7	А
		East	26	В
	AM	North	31	С
		West	23	В
		All	26	В
		East	14	А
Goodall Street/	5.4	North	24	В
Dunmore Street	PIVI	West	14	А
		All	18	В
		East	14	А
	Sat	North	20	В
		West	15	В
		All	16	В
	AM	South	20	В
		East	23	В
		North	16	В
		All	20	В
	PM	South	16	В
Pendle Way/ Dunmore		East	20	В
Street		North	15	В
		All	18	В
	Sat	South	10	A
		East	22	В
		North	12	A
		All	15	В

The priority controlled intersections of Jones Street/ Rogers Street and Jones Street/ Oatlands Street are currently operating with little to no delay (less than 14 seconds) and an average Level of Service A during all peak hours. This signifies good intersection operation.

The roundabout controlled intersections of Dunmore Street/ Jones Street and Jones Street/ Smith Street are also operating with minimal delay (less than 20 seconds) and an average Level of Service A/ B during all peak hours, constituting a good level of intersection operation.



The signalised intersections of Dunmore Street/ Goodall Street and Dunmore Street/ Pendle Way are both currently operating with minimal but acceptable delay (35 seconds or less). Both intersections have an average Level of Service B, indicating spare capacity.

2.4 Public Transport

2.4.1 Rail

Pendle Hill Station is located approximately 500m north-west of the subject site, equivalent to an 8-10 minute walk. The station is served by the North Shore and Western Line, and the Cumberland Line.

The North Shore and Western Line travels from Richmond and Emu Plains through Parramatta and the Sydney CBD to North Sydney and Chatswood. During the AM and PM peak, services run every 15 minutes. Outside of the peak services run every 30 minutes.

The Cumberland Line travels from Blacktown through Parramatta and Liverpool to Campbelltown. During the AM and PM peak, services run every 15 minutes. Outside of the peak services run every 30 minutes.

Combined, these train services run every 5-10 minutes in the peak and every 20-30 minutes in the off-peak in both directions. Transferring trains at Parramatta Station allows for more frequent trips between Pendle Hill and major destinations such as the Sydney CBD and Liverpool.

2.4.2 Bus

Local bus services are operated by Hillsbus and include the following routes:

- Route 700: Every 15 minutes during weekday AM/ PM peak and every hour off-peak (including weekends). Runs along Smith Street between Blacktown Station and Parramatta Station via Girraween and Pendle Hill.
- Route 705: Every 30 minutes during weekday AM/ PM peak and every hour off-peak (including weekends). Runs along Dunmore Street and Pendle Way between Seven Hill, Toongabbie, Pendle Hill, Wentworthville and Parramatta.
- Route 708: One service in the morning and one in the afternoon. Runs along Dunmore Street and Pendle Way, connecting retirement villages in Parramatta, Wentworthville and Pendle Hill.

2.5 Pedestrian Infrastructure

Pedestrian paths are located as follows:

- Dunmore Street 1.2m wide footpath on both sides
- Jones Street 1.2m wide footpath on the western side only
- Pendle Way 1.2m wide footpath on both sides, widening to approximately 3m in the town centre
- Smith Street 1.2m wide footpath on the northern side only
- Rogers Street 1.2m wide footpath on the southern side only
- Oatlands Street 1.2m wide footpath on the southern side only.

The northern and western arms of the Dunmore Street/ Goodall Street intersection and the eastern and southern arms of the Dunmore Street/ Pendle Way intersection have signalised pedestrian crossings.



Pedestrian refuges are provided on all the arms of the roundabout intersections of Smith Street/ Jones Street and Dunmore Street/ Jones Street.

There are no other pedestrian crossings in the vicinity of the site. Additional crossing facilities could be provided to help encourage increased pedestrian activity.

2.6 Cycle Infrastructure

There are no dedicated cycling facilities within the immediate vicinity of the subject site, however many of the local streets are suitable for mixed traffic. Children under the age of 12, and accompanying adults can of course legally cycle on footpaths.

The 2009 Holroyd Bike Plan, shown in Figure 2.11, indicates that on-road bicycle facilities are proposed along Dunmore Street and Pendle Way. These are likely be mixed traffic facilities shared with parked vehicles.

Four secure bicycle lockers are currently available at Pendle Hill Station.



Figure 2.11: Holroyd Bike Plan

2.7 Local Car Sharing Initiatives

There are currently no local car sharing initiatives operating near the subject site.



3. Development Proposal

3.1 Land Uses

The revised Concept Master Plan, shown in Figure 3.1, includes approximately 1,600 new dwellings and 6,000m² of retail floor space, including approximately 4,000m² of supermarket and 2,000m² of specialty retail. The final dwelling numbers/mix as well as retail floor areas will be confirmed at a later stage.

Figure 3.1: Site Layout



Source: Roberts Day dated 26 April 13 (Drawing No. RD3 313).

3.2 Vehicle Access

The revised Concept Master Plan will seek to provide vehicle access via both of the site's road frontages, namely Dunmore Street and Jones Street. The provision of multiple site access roads with supporting internal road linkages will assist the general distribution and dispersion of site generated traffic to the external road network.



The exact location of accesses is not proposed to be fixed at this point as the location will be dependent upon a number of variables (e.g. heritage buildings). Whilst it was the intention to use the existing single access T-intersection on Dunmore Street and Jones Street, it is unlikely that a simple tee intersection would be able to operate satisfactorily on Dunmore Street. Furthermore this access is close to the Dunmore Street/Jones Street roundabout.

Consequently, in Section 4.6, scenarios have been tested which show that access to the site can be satisfactorily achieved on the Dunmore and Jones Street frontages, this report does not intend to finalise the form and location of these as this stage as they may need to amended during the development application. However the traffic analysis does show, for the purposes of rezoning the site, that satisfactory accesses can be achieved along the site frontages.

3.3 Car Parking

On-site car parking provision would need to be provided in accordance with the goals and objectives as set out in the City of Holroyd LEP 2013 and DCP 2013. Essentially the objectives of the current DCPs are to ensure that there is sufficient on-site car parking to accommodate the site's parking demands.

Figure 3.2 has been extracted from the City of Holroyd DCP 2013 and provides the minimum and maximum car parking requirements for residential and retail uses.

Residential				
Use	Measure	Minimum Spaces Required	Maximum Spaces Required	
Attached dwellings and Small lot dwelling houses (<300m ² or 8m or less width)	Per dwelling	I	2 (max. I covered)	
Dwelling houses (other than on small lots), semi detached dwellings, dual occupancies.	Per dwelling	2 (min. I covered)	n/a	
	Bedroom per dwelling: Studio / I bedroom	I	1.5	
Multi dwelling housing	2 bedroom	Ι	2	
	3 bedroom	1.2	2	
	4+ bedroom	1.5	2	
	Visitor / dwelling	0.2	0.5	
Residential flat buildings, dwellings in B1, B2	Bedroom per dwelling: Studio / I bedroom	0.8	I.	
and B6 business zones (including shop top	2 bedroom	1	1.5	
housing)	3 bedroom	1.2	2	
	4+ bedroom Visitor / dwelling	1.5	0.5	
	Bedroom per dwelling:	0.8		
Dwellings in mixed use development in B4	2 bedroom		1.2	
Mixed Use zone (including shop top housing)	3 bedroom	I	1.2	
	4+ bedroom	1.2	1.5	
	Visitor / dwelling	0.2	0.2	
	Retail & Commercial			
Use	Measure	Minimum Spaces Required	Maximum Spaces Required	
Commercial (including retail premises,	Ground Floor Leasable			
business premises and office premises) - B4	GFA	1 50 3	l per 15m²	
zone	Above Ground Floor	I per 50m ²		
	Leasable GFA			
Commercial (including retail premises,	Ground Floor - Leasable	1 20 3		
business premises and office premises) - B2	GFA	I per 20m ²	I per 15m ²	
zones in: * Wentworthville * Pendle Hill * Toongabbie * Guildford	Above Ground Floor - Leasable GFA	l per 40m²	l per 20m²	
Commercial (including retail premises,	Ground Floor - Leasable GFA	l per 20m²	l per 10m²	
other BI, B2 and B6 zoned areas	Above Ground Floor - Leasable GFA	l per 40m²	l per 15m²	
Neighbourhood shop	leasable GFA	I space per 30m ²	n/a	

Figure 3.2: City of Holroyd DCP 2013 – Car Parking Requirements

Extract from City of Holroyd DCP 2013

Based on the above, the car parking provisions for the proposed mixed use development would need to provide in the order of 2,040 spaces with a breakdown by use as follows:

- 1,600 spaces for Residential
- 320 for Residential Visitors
- 120 for Retail.



The demands of the development on the site also need to be considered with the broader state and local government transport objectives where non-private motor vehicle modes of travel are to be encouraged.

With the above in mind, and given the proposed rezoning of the site to Mixed Use, the Concept Master Plan envisages the following general on-site car parking provisions:

- Residential at 1 space / dwelling = 1,600 residential parking spaces
- Visitor parking at 1 space / 5 dwellings = 320 residential visitor parking spaces
- Retail at 1 space / 44m² GLFA = 137 retail parking spaces.

As such the envisaged provision of 2,057 car parking spaces accords with the minimum City of Holroyd DCP 2013 car parking requirements.

It is also expected that the supermarket and associated specialty stores would operate complementary to residential development, with a significant proportion of the trips generated internally/locally. Consequently, the parking provision at the site will be required for local/pass by facility rather than for a destination shopping centre.

Notwithstanding the above, the exact on-site parking provision will need to be defined once further details regarding the type of retail, commercial and residential land uses are proposed on the site.

3.4 Pedestrian Facilities

A network of new pedestrian paths connecting to the surround area would be provided as part of the proposed development. Additional details will be provided at the development application stage.

3.5 Bicycle Facilities

Cycle paths will be provided through the site and bicycle parking will be provided in accordance with the requirements of the City of Holroyd DCP.

3.6 Loading Areas

Loading areas for the proposed supermarket and specialty retail shops, as well as service vehicle access and refuse storage areas are to be provided in accordance with the City of Holroyd DCP and the relevant Australian Standards.

The required provisions will be determined at a later stage.

3.7 Consistency of Development Proposal with Holroyd Centres Strategy

The Holroyd Residential Centres Strategy Transport Review prepared by Stapleton Transportation and Planning Pty Ltd (STAP) identifies that:

- Pendle Hill provides good opportunities for the retention and enhancement of the currently surveyed use of non-car travel, and is well situated to provide high and medium density residential development.
- STAP is of the opinion that it would not be unwarranted for Council to investigate further expansion of the high density zone (or provision of a medium density zone)



south of the station to 400m at least if additional residential capacity is required in the future.

In terms of the provision of infrastructure to accommodate the future growth in population, previous discussions with Sydney Trains indicated that as the population grows in a given area and demand subsequently grows for rail services, the level of services to particular stations can be adjusted accordingly.

With regard to parking, the transport consultant review concluded that "total parking demands within Holroyd, and within the transit centres, will be lower under a high density residential strategy than under a dispersed strategy". According to their research and analysis "High density residential development in close proximity to transit centres (unlike dispersed medium density) reduces commuter parking demand, as residents can walk to public transport services rather than have to drive.....their commuter parking is provided in the basement parking levels below their dwelling. Non-residential development (i.e. shops and businesses) within the transit centres also generates a lower parking demand, as a higher percentage of visitors (staff, customers etc.) can walk/cycle or use public transport to access the development".

With a dispersed strategy (applied to either the new non-residential or residential development) those same customers simply cannot walk/cycle the greater distance and as such car trip and car parking demand is higher.


4. Traffic Impact Assessment

The traffic impact assessment presented in this section was prepared on the basis of an earlier development profile of 1,679 dwellings. Consequently, the modelling provides a conservative approach.

4.1 Traffic Generation

Traffic generation estimates for the proposed development have been sourced from the RMS *Guide to Traffic Generating Development* (including the recently issued Technical Direction TDT 2013/04a). The RMS Guide specifies different rates having consideration for a number of parameters including the size and location of the development. The document recorded the following traffic generation at the surveyed sites (which general comprise a mixture of unit sizes but with the majority being 2 bed apartments).

		RMS Updated Traffic Surveys (2012)#							
	Site 1 – St Leonards	Site 2 - Chatswood	Site 3 - Cronulla	Site 4 - Rockdale	Site 5 - Parramatta	Site 6 – Liberty Grove	Site 7 - Strathfield	Site 10 - Pyrmont	Sydney Metropolit an Area (Average)
No. of Units	70	129	28	234	83	64	31	131	96
No. of Parking Spaces	97	206	18	260	108	93	30	199	126
Parking Provision Ratio (spaces/unit)	1.39	1.60	0.64	1.11	1.30	1.45	0.97	1.52	1.31
Peak hour vehicle trips per unit	0.14(AM) 0.07(PM)	0.14(AM) 0.12(PM)	0.07(AM) 0.11(PM)	0.32(AM) 0.18(PM)	0.27(AM) 0.12(PM)	0.28(AM) 0.41(PM)	0.10(AM) 0.06(PM)	0.18(AM) 0.10(PM)	0.19 (AM) 0.15 (PM)

 Table 4.1:
 Traffic generation of High Density Dwellings

The sites surveyed within Sydney Metropolitan Area included sites within close proximity to the Sydney CBD with excellent access to public transport and retail and commercial precincts. As such, although the development is located in close proximity to public transport, traffic generation rates towards the higher end of the spectrum are anticipated. We have therefore used some of the highest recorded rates in the study (i.e. those at Rockdale), these rates being:

- weekday AM peak hour 0.32 vehicles per dwelling two-way
- weekday PM peak hour 0.18 vehicles per dwelling two-way
- Saturday peak hour 0.23 vehicles per dwelling two-way.

The revised Concept Master Plan also includes approximately 6,000m² of retail floor area, including approximately 4,000m² of supermarket.

The RMS Guide, in Section 3.6.1, indicates that a supermarket of this size would generate around 155 and 147 two-way vehicle movements (per 1,000m²) in the Thursday PM and Saturday peak hours respectively, and specialty stores around 46 and 107 two-way vehicle movements (per 1,000m²) in the Thursday PM and Saturday peak hours respectively.

For the purpose of this assessment, it has been assumed that during the AM peak hour the traffic movements for the retail component would be around 50% of that generated during the PM peak hour.

It is also envisaged that the supermarket and associated specialty stores will operate complementary to residential development, with a limited number of trips generated externally. Retail analysis suggests that approximately 40% of retail activity in mixed use developments is generated by the residential component of the site. However, a conservative reduction of 25% has been applied to reflect this characteristic of the development - suggesting that up to 75% of trips could be generated externally (which again is believed to be a conservative figure).

Based on the above, estimates of external peak hour traffic volumes resulting from the propos	sal
are set out in Table 4.2.	

			1 /				
		Generation Rate			Movements/hr.		
Use	Units / GFA	AM	PM	Sat	AM	PM	Sat
Residential							
Dwellings	1,679	0.32 per dwelling	0.18 per dwelling	0.23 per dwelling	537	302	386
Retail							
Supermarket	4,000m ²	58 per 1,000m ²	116 per 1,000m ²	110 per 1,000m ²	232	464	440
Specialty retail	2,000m ²	17.5 per 1,000m ²	35 per 1,000m ²	80 per 1,000m ²	35	70	160
				r	i	r	
Total					804	836	986

Table 4.2: Traffic Generation Estimates (External Trips)

Table 4.2 indicates that the site could potentially generate approximately 800 external vehicle movements in the AM peak hour, 840 external vehicle movements in the PM peak hour and 1,000 external vehicle movements in the Saturday midday peak hour.

This compares to the 800 peak hour trips that could be generated by redeveloping the site to accommodate a higher FSR whilst maintaining a similar industrial use, as detailed in Section 2.2.1.

4.2 Distribution and Assignment

The directional distribution and assignment of traffic generated by the proposed development will be influenced by a number of factors, including the:

- configuration of the arterial road network in the immediate vicinity of the site
- existing operation of intersections providing access between the local and arterial road network
- distribution of households both within the site and in the vicinity of the site
- surrounding employment centres, retail centres and schools in relation to the site
- configuration of access points to the site.

Having considered the above, for the purposes of estimating vehicle movements, the directional split of traffic (i.e. the ratio between the inbound and outbound traffic movements) has been assumed as follows:

- Residential: 20% inbound/ 80% outbound during the Thursday AM peak, and 40% inbound/ 60% outbound during the Thursday PM peak.
- Retail: 70% inbound/ 30% outbound during the Thursday AM peak, and 60% inbound/ 40% outbound during the Thursday PM peak.
- During the Saturday midday peak, the directional split of both retail and residential traffic has been assumed as 50% outbound/ 50% inbound.

The splits are based on results from traffic surveys undertaken by Halcrow to update the RMS Guide to Traffic Generating Developments for shopping centres.

As stated in Section 3.2, the traffic distribution and assignment was undertaken based on one access on Dunmore Street and one access on Jones Street.



The NSW Governments' Bureau of Transport Statistics website was used to undertake where employed residents from Pendle Hill work. Mode of travel had also been considered with only vehicle travel assessed. The distribution of residential traffic was determined based on the top six destinations, factoring in the portion that drive, and has been summarised in Table 4.3.

Employment Region	Where residents work	Portion that drive to work	Distribution Factor
Parramatta	36%	17%	40%
City	22%	4%	5%
Blacktown	14%	20%	19%
Auburn	14%	19%	18%
Ryde	7%	18%	9%
Baulkham Hill	6%	22%	8%
	100%	100%	100%

Table 4.3: Journey to Work Assessment

Therefore based on the above the anticipated traffic distribution for the residential component is summarised in Figure 4.1.



Figure 4.1: Traffic Distribution – Residential Component

Figure 4.1 illustrates that the employed residents that work east of Pendle Hill would either travel between the Great Western Highway and the development by either Dunmore Street (15%), Smith Street (20%) or Jones Street (30%). Employed residents that work west of Pendle Hill would either travel via Pendle Way (5%) or Jones Street (15%). All traffic towards the north (15%) would use Dunmore Street. Traffic using Dunmore Street to/ from Cumberland Highway would access the development via either the Dunmore Street or Jones Street access (50/50 split).



For the retail component, the anticipated traffic distribution was determined based on the residential and retail in proximity to the development and is presented in Figure 4.2.



Figure 4.2: Traffic Distribution - Retail Component

Figure 4.2 illustrates that 70% of retail traffic would access the development via the Dunmore Street access and 30% via the Jones Street access. This was determined based on the retail component being located at the north end of the development.

4.3 Future Impacts

To consider the likely traffic conditions on the surrounding road network under a 10 year horizon (dependant on when the development is likely to be constructed), a "per annum" linear growth factor was applied to the background traffic.

The growth of traffic surrounding the site is influenced by a range of factors including nearby developments and regional traffic conditions. Given that the surrounding area is well established, future developments are likely to be 'infill' developments of existing sites, a 1% per annum linear growth rate was applied to the background traffic.

For the purpose of this assessment it was assumed that the development would be completed by 2017, hence modelling was undertaken for the Year 2027.

In addition, two 10 year horizon scenarios have been assessed, a 'without development' scenario to understand the likely traffic conditions in 2027 without the development; and a 'with development' scenario to understand the potential impacts the development has on the road network.



Based on the above, Figure 4.3, Figure 4.4 and Figure 4.5 have been prepared to show the anticipated Year 2027 background traffic volumes and the estimated increase in turning movements (shown in red on the figures) in the vicinity of the subject property following full site development.



Figure 4.3: Thursday AM Peak Hour Site Generated Traffic Volumes





Figure 4.4: Thursday PM Peak Hour Site Generated Traffic Volumes





Figure 4.5: Saturday Midday Peak Hour Site Generated Traffic Volumes



4.4 Traffic Impact

Following the distribution and assignment of development traffic and application of growth to background traffic, the study intersections were reanalysed using SIDRA INTERSECTION.

Table 4.4 summarises the SIDRA results of the Year 2027 'without development' and 'with development' scenarios in terms of average delay and level of service.

The full results from the SIDRA analysis are presented in Appendix A of this report.

			'Without De	velopment'	'With Development'		
Intersection	Peak	Leg	Average Delay (sec)	Level of Service (LOS)	Average Delay (sec)	Level of Service (LOS)	
		South	2	NA	2	NA	
	0.14	East	11	А	12	А	
	Alvi	North	0	NA	0	NA	
		All	1	NA	1	NA	
		South	2	NA	2	NA	
Jones Street/	DM	East	11	А	11	А	
Rogers Street	PIVI	North	0	NA	0	NA	
		All	1	NA	2	NA	
		South	1	NA	2	NA	
	<u> </u>	East	9	А	10	А	
	Sat	North	0	NA	0	NA	
		All	1	NA	2	NA	
		South	1	NA	4	NA	
		East	9	А	20	В	
	AM	North	0	NA	0	NA	
		All	1	NA	2	NA	
	PM	South	2	NA	4	NA	
Jones Street/		East	10	А	27	В	
Oatlands Street		North	0	NA	0	NA	
		All	2	NA	4	NA	
	Sat	South	2	NA	4	NA	
		East	9	А	24	В	
		North	0	NA	0	NA	
		All	1	NA	3	NA	
		South	7	А	8	А	
		East	8	А	11	А	
	AM	North	9	А	18	В	
		West	8	А	10	А	
		All	8	А	13	А	
		South	8	А	10	А	
		East	7	А	9	А	
Jones Street/	PM	North	7	А	7	А	
smith street		West	7	A	9	А	
		All	7	А	9	А	
		South	7	А	8	А	
		East	7	A	10	A	
	Sat	North	7	A	7	А	
		West	7	А	9	А	
		All	7	А	8	А	

Table 4.4:	Year 2027 'Without Development	' and 'With Development'	scenarios – SIDRA Results
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			'Without De	velopment'	'With Development'		
Intersection	Peak	Leg	Average Delay (sec)	Level of Service (LOS)	Average Delay (sec)	Level of Service (LOS)	
		South	10	А	12	А	
		East	8	А	9	А	
	AM	North	18	В	24	В	
		West	8	А	27	В	
		All	9	А	20	В	
		South	15	В	23	В	
Jones Street/		East	11	А	16	В	
Dunmore	PM	North	11	А	12	А	
Street		West	8	А	8	А	
		All	11	А	14	А	
		South	10	А	11	А	
		East	7	А	9	А	
	Sat	North	11	А	12	А	
		West	7	А	7	А	
		All	8	А	9	А	
	AM	East	34	С	29	С	
		North	46	D	77	F	
		West	35	С	57	E	
		All	38	С	56	D	
	PM	East	18	В	20	В	
Goodall Street/		North	23	В	27	В	
Dunmore		West	15	В	17	В	
Street		All	19	В	21	В	
		East	15	В	16	В	
		North	20	В	24	В	
	Sat	West	15	В	17	В	
		All	16	В	19	В	
		South	23	В	24	В	
		East	26	В	27	В	
	AM	North	19	В	23	В	
		All	23	В	24	В	
		South	18	В	23	В	
Pendle Way/		East	22	В	23	В	
Dunmore Street	PM	North	15	В	17	В	
01001		All	19	В	22	В	
		South	11	A	15	В	
		East	23	В	23	В	
	Sat	North	13	А	13	А	
		All	16	В	17	В	

Table 4.4: Year 2027 'Without Development' and 'With Development' scenarios - SIDRA Results (cont.)



The following conclusions are made from the modelling results:

- The priority controlled intersection of Jones Street/Rogers Street would continue to operate with a Level of Service A.
- The priority controlled intersection of Jones Street/ Oatlands Street would experience some minor delay in the peaks for vehicles entering from Oatlands Street (comparative to 'without development' conditions), however the intersection would maintain an acceptable operating conditions with an average Level of Service B.
- The roundabout controlled intersection of Jones Street/ Smith Street would maintain good operating conditions with an average Level of Service A/ B.
- The roundabout controlled intersection of Jones Street/ Dunmore Street would experience some minor additional delay in the Thursday AM peak on the west approach (<30 seconds) and Thursday PM peaks on the east approach (<30 seconds), compared to 'without development' conditions. The intersection would maintain to operate with an average Level of Service B during these peak hours which is considered to be a good level of service. During the Saturday midday peak hour the intersection continues to operate with an average Level of Service A.
- The signalised intersection of Dunmore Street/ Goodall Street would experience additional delay and queuing during the Thursday AM peak. Overall, the intersection would operate with an average Level of Service D which is considered to be approaching capacity. During the Thursday PM and Saturday midday peaks the intersection would continue to operate with an average Level of Service B.
- The signalised intersection of Dunmore Street/ Pendle Way would continue to operate with a Level of Service B with some minor additional delay experienced. This is considered to be a good level of service.

4.5 Mitigating Measures

There are a number of options which could be progressed to improve operating conditions at the intersection of Dunmore Street/ Goodall Street. These include:

- altering signal phasings to optimise vehicle movements;
- altering or removing parking provisions along Dunmore Street (east) and Goodall
 Street to provide increased vehicle capacity on approach to and departure from the intersection; and
- providing a residential travel plan to minimise the number of peak hour car trips generated by the site (detailed further below).

4.6 Modelling of Accesses

As stated in Section 3.2, this section provides an assessment of a number of potential access options for consideration during the Development Application once the site layout is at a more detailed stage.

Figure 4.6 to Figure 4.10 present the access configurations assessed using SIDRA INTERSECTION and the results are provided in Table 4.5 and Table 4.6 for the Dunmore Street and Jones Street accesses respectively.





Figure 4.8: Dunmore Access – Roundabout-Controlled



Figure 4.10: Jones Access – Priority-Controlled All Movements



Figure 4.7: Dunmore Access – Priority-Controlled Left in/ Left Out



Figure 4.9: Dunmore Access – Signalised-Controlled



Intersection	Peak	Leg	Average Delay (sec)	Level of Service (LOS)
		South	146	F
		East	0	NA
	AIVI	West	11	NA
		All	18	NA
		South	36	С
Priority-Controlled	DM	East	1	NA
(All Movement)	PIVI	West	15	NA
		All	10	NA
		South	25	В
	C = t	East	1	NA
	Sat	West	9	NA
		All	9	NA
		South	14	А
	0.04	East	1	NA
	AM	West	0	NA
		All	1	NA
	PM	South	21	В
Priority-Controlled		East	2	NA
(Left-In/ Left-Out)		West	0	NA
		All	2	NA
		South	17	В
		East	3	NA
	Sat	West	0	NA
		All	3	NA
		South	13	А
	0.5.4	East	6	А
	AIVI	West	7	А
		All	7	А
		South	12	А
	DM	East	5	А
Roundabout	PIVI	West	5	А
		All	6	А
		South	10	А
	C - 1	East	5	А
	Sat	West	6	А
		All	7	А

 Table 4.5:
 Dunmore Street Access Options Testing – SIDRA Results

	-	-		
		South	54	D
	0.54	East	4	А
	AIVI	West	2	А
		All	7	А
	PM	South	46	D
		East	8	А
signalised		West	8	А
		All	12	А
	Sat	South	38	С
		East	12	А
		West	15	В
		All	18	В

able 4.5:	Dunmore Street	Access Options	Testing -	SIDRA Results	(cont.)
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Based on options testing indicated above, for the Dunmore Street access, the following conclusions are made:

- An all movement priority-controlled intersection would operate at a Level of Service F during the Thursday AM peak hour (worst case). This is a result of the right-turn movement from the access would experiencing significant delays and queuing due to high volumes of through movement on Dunmore Street reducing the opportunities to undertake the manoeuvre. Consequently, this option has been discounted.
- A left-in/ left-out intersection would operate at a Level of Service B during the Thursday PM and Saturday midday peak hours (worst cases). This option was tested assuming all proposed right-turn development traffic from Dunmore Street into the development would continue to use the access via undertaking a U-turn manoeuvre at roundabout at Jones Street. However, due to the proximity of the intersection to the Dunmore Street/ Jones Street roundabout, this has been discounted.
- A roundabout-controlled intersection would operate at a Level of Service A during all peak hours.
- A signalised-controlled intersection would operate at a Level of Service B during the Saturday midday peak hour (worst case).

Intersection	Peak	Leg	Average Delay (sec)	Level of Service (LOS)
		South	2	NA
	0.5.4	North	2	NA
	AIVI	West	16	В
		All	7	NA
		South	3	NA
Priority-Controlled	DNA	North	3	NA
(All Movement)	PIVI	West	17	В
		All	6	NA
		South	4	NA
	Sat	North	3	NA
		West	15	В
		All	7	NA
	AM	South	2	NA
		North	2	NA
		West	15	В
		All	7	NA
		South	3	NA
Priority-Controlled	DNA	North	3	NA
(All Movement) [1]	PIVI	West	16	В
		All	6	NA
		South	4	NA
	Set	North	3	NA
	ડતા	West	15	В
		All	8	NA

Table 4.6	Iones Street Access	Options Testing -	SIDRA Results
10010 1.0.	301103 301000 71000033	options resting	JIDIN (RCJultj

[1] If Dunmore Street access was Left in/Left out only. As such, all right-turn movements from the Dunmore access would be relocated.

Based on options testing above for an access on Jones Street, the following conclusions are made:

- An all movement priority-controlled intersection would operate at a Level of Service B during all peak hours.
- Should the Dunmore Street access be a left-in/ left-out arrangement, the additional traffic resulting from the inability to undertake a right-turn into Dunmore Street would not impact on the intersection performance and a Level of Service B during all peak hours would be maintained.

In summary, this shows that for the purposes of rezoning, adequate access can be provided on both the Dunmore and Jones Street frontages. The likely preference for access from Dunmore Street would be for a roundabout as this would also introduce an element of traffic calming on Dunmore Street and there are a number of locations where such an intersection could be provided without compromising access into properties on the northern side of Dunmore Street. Furthermore, the proposed access would allow closure of the existing access which is located close to the Jones Street/ Dunmore Street roundabout which would provide additional road safety benefits.

The access onto Jones Street would be likely to be in the form of a priority tee intersection.



4.7 Impact on Additional Intersections

Further to the original analysis, Roads and Maritime Services requested additional consideration be given to the impacts of the development on a number of intersections located beyond the seven intersections modelled by GTA. These intersections have been shown on Figure 4.11.

The overall anticipated traffic volumes added to each of the intersections are presented in Figure 4.11.



Figure 4.11: Traffic Volumes at Additional Intersections

Of the intersections considered, it is clear that the Pendle Way/ Gilba Street and Pendle Way/ Great Western Highway intersections would experience insignificant impacts with a maximum of one vehicle every three minutes.

Similarly, the Wentworth Avenue/ Goodall Street intersection would experience a maximum of one vehicle every one minute during the Saturday peak.

The intersections on the Cumberland Highway would experience more than a vehicle every minute entering Cumberland Highway during the Thursday AM peak, at other times they would experience less than one vehicle every one minute.

Similarly, with respect to the Jones Street intersection with the Great Western Highway, it is noted that the increase in traffic would be between one to two minutes per vehicle, except during the Thursday AM peak with a vehicle turning left into the highway every 30 seconds.



In summary, it can be seen that the additional traffic at each of the intersections offers a very modest increase. In terms of the two-way flows on the Cumberland highway and the Great Western Highway, the additional traffic would be negligible and would probably be within the daily variation in traffic. The additional traffic on the side road arms is generally less than one a minute and would be unlikely to affect the performance of the intersections significantly.

Due to these minor increases in flow, it has been considered that the modelling of the intersections is an excessive requirement for rezoning as the development is unlikely to trigger the need for any improvements at these intersections. If necessary, however, this additional intersection modelling could be undertaken at development application stage.

4.8 Residential Travel Plan

The use of such plans has been accepted at such sites as Harold Park in Sydney as a means of reducing the peak hour traffic impacts of development. A number of measures can be provided to encourage sustainable travel, and hence reduced car use, such as:

- Creation of street networks and associated cycle ways, footpaths and links to encourage cycling and walking.
- Provision of a Transport Access Guide which would be given to every new occupant of dwellings.
- Provision of public transport information boards to make residents and visitors more aware of the alternative transport options available to them. The format would be based upon the Transport Access Guide.
- Provision of a free public transport tickets for the initial occupation of the dwellings so that residents would be encouraged to make public transport their modal choice from the day they moved into their new dwelling.
- Provision of half yearly membership to a car club which could have dedicated cars and dedicated parking spaces reasonably close to the proposed development.
- Providing properties with high quality telecommunication points which will provide residents with the opportunity to work at home and to reduce the need to travel.
- Provision of bicycle parking spaces both for residents and for visitors to the site.
- Provision of a half yearly newsletter to residents to promote local travel initiatives.

All residents could be given this travel information and any associated membership in the owners pack for new residents.



5. Conclusion

Based on the analysis and discussions presented within this report, the following conclusions are made:

- A Concept Master Plan has been developed for the site, incorporating approximately 1,600 new dwellings and approximately 6,000m² of retail floor space, including a supermarket.
- ii The site is within walking distance to Pendle Hill Station and bus stops are located on the surrounding streets. Note: It is generally accepted that residents are willing to walk up to 800m to a rail station as opposed to only 400m to a bus stop.
- iii The envisaged provision of 2,057 on-site car parking spaces accords with the minimum City of Holroyd DCP 2013 car parking requirements.
- iv The site is expected to generate approximately 800-850 vehicle movements in the AM and PM peak hour and 1,000 vehicle movements in the Saturday midday peak hour.
- v A potential re-use of the site for industrial purposes with an FSR comparable with adjacent development would be likely to generate at least 800 trips in the peak hours
- vi Other than at the intersection of Dunmore Street/ Goodall Street, there appears to be adequate capacity in the surrounding road network to cater for the traffic generated by the proposed development.
- vii Operation of the signalised intersection at Dunmore Street/ Goodall Street could be improved by altering signal phasings to optimise vehicle movements, altering or removing parking provisions along Dunmore Street (east) and Goodall Street, and/ or reviewing the number or configuration of apartments/ commercial floor space to reduce overall vehicle flows expected.
- viii The anticipated increase in traffic resulting from the development at each of the additional intersections that RMS asked to be considered is expected to be very modest.
- ix In terms of the two-way flows on the Cumberland Highway and the Great Western Highway, the additional traffic would be negligible and would probably be within the daily variation in traffic.
- x Due to these minor increases in flow, it has been considered that the modelling of the intersections is an excessive requirement for rezoning as the development is unlikely to trigger the need for any improvements at these intersections. If necessary however, the intersection modelling could be undertaken at development application stage.
- xi As the exact location of entrances and exits to the site along Jones Street and Dunmore Street cannot be fixed at this time, a range of access options has been presented and the analysis shows that adequate access to the site can be achieved.
- xii The development proposal is consistent with the Holroyd Residential Centres Strategy Transport Review which identifies that
 - Pendle Hill provides good opportunities for the retention and enhancement of the currently surveyed use of non-car travel, and is well situated to provide high and medium density residential development.
 - It would not be unwarranted for Council to investigate further expansion of the high density zone (or provision of a medium density zone) south of the station to 400m at least if additional residential capacity is required in the future.

In conclusion, the proposed redevelopment would be able to proceed without having a significant adverse impact on the performance of the road network in the vicinity of the site.

Appendix A



Appendix A



SIDRA INERSECTION Results

13S1210100 Jones Street-Rogers Street Giveway / Yield (Two-Way)

Movem	ovement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h		
South: J	ones S	Street											
2	Т	242	2.0	0.131	1.3	LOS A	1.0	6.8	0.43	0.00	44.9		
3	R	5	2.0	0.131	8.1	LOS A	1.0	6.8	0.43	0.89	43.0		
Approac	h	247	2.0	0.131	1.5	NA	1.0	6.8	0.43	0.02	44.9		
East: Rogers Stree		Street											
4	L	5	2.0	0.019	10.4	LOS A	0.1	0.5	0.48	0.60	39.9		
6	R	5	2.0	0.019	10.8	LOS A	0.1	0.5	0.48	0.75	39.8		
Approac	h	11	2.0	0.019	10.6	LOS A	0.1	0.5	0.48	0.67	39.8		
North: Jo	ones S	treet											
7	L	5	2.0	0.142	6.5	LOS A	0.0	0.0	0.00	0.91	43.3		
8	Т	267	2.0	0.142	0.0	LOS A	0.0	0.0	0.00	0.00	50.0		
Approac	:h	273	2.0	0.142	0.1	NA	0.0	0.0	0.00	0.02	49.9		
All Vehic	les	531	2.0	0.142	1.0	NA	1.0	6.8	0.21	0.03	47.2		

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Project: \\GTA-SYD-SS1\Project_Files\13S1200-1299\13S1210100 Bonds Pendle Hill Amended Report\Modelling \140314sid - 13S1210100 Bonds Spinning Mills - Existing Conditions.sip 8000056, GTA CONSULTANTS, ENTERPRISE

13S1210100 Jones Street-Rogers Street Giveway / Yield (Two-Way)

Movem	lovement Performance - Vehicles													
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h			
South: J	lones S	street												
2	Т	255	2.0	0.143	1.3	LOS A	1.0	7.4	0.44	0.00	44.8			
3	R	12	2.0	0.143	8.1	LOS A	1.0	7.4	0.44	0.88	43.0			
Approac	h	266	2.0	0.143	1.6	NA	1.0	7.4	0.44	0.04	44.8			
East: Rogers Stree		treet												
4	L	12	2.0	0.030	9.8	LOS A	0.1	0.7	0.45	0.62	40.4			
6	R	6	2.0	0.030	10.2	LOS A	0.1	0.7	0.45	0.77	40.2			
Approac	h	18	2.0	0.030	10.0	LOS A	0.1	0.7	0.45	0.67	40.3			
North: J	ones S	treet												
7	L	6	2.0	0.142	6.5	LOS A	0.0	0.0	0.00	0.91	43.3			
8	Т	266	2.0	0.142	0.0	LOS A	0.0	0.0	0.00	0.00	50.0			
Approac	h	273	2.0	0.142	0.1	NA	0.0	0.0	0.00	0.02	49.8			
All Vehic	cles	557	2.0	0.143	1.2	NA	1.0	7.4	0.22	0.05	46.9			

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Project: \\GTA-SYD-SS1\Project_Files\13S1200-1299\13S1210100 Bonds Pendle Hill Amended Report\Modelling \140314sid - 13S1210100 Bonds Spinning Mills - Existing Conditions.sip 8000056, GTA CONSULTANTS, ENTERPRISE 13S1210100 Jones Street-Rogers Street Giveway / Yield (Two-Way)

Movem	Average level of Oracle Provide Provide Average level of Oracle of Oracle Provide Provide Average													
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h			
South: J	ones S	treet												
2	Т	162	2.0	0.089	0.9	LOS A	0.6	4.2	0.36	0.00	45.7			
3	R	5	2.0	0.089	7.7	LOS A	0.6	4.2	0.36	0.88	43.0			
Approac	h	167	2.0	0.089	1.1	NA	0.6	4.2	0.36	0.03	45.6			
East: Rogers Stree		treet												
4	L	12	2.0	0.024	8.5	LOS A	0.1	0.6	0.37	0.58	41.4			
6	R	6	2.0	0.024	8.9	LOS A	0.1	0.6	0.37	0.71	41.3			
Approac	h	18	2.0	0.024	8.7	LOS A	0.1	0.6	0.37	0.63	41.4			
North: Jo	ones St	reet												
7	L	14	2.0	0.110	6.5	LOS A	0.0	0.0	0.00	0.89	43.3			
8	Т	198	2.0	0.110	0.0	LOS A	0.0	0.0	0.00	0.00	50.0			
Approac	h	212	2.0	0.110	0.4	NA	0.0	0.0	0.00	0.06	49.5			
All Vehic	cles	397	2.0	0.110	1.1	NA	0.6	4.2	0.17	0.07	47.4			

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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13S1210100 Jones Street-Oatlands Street Giveway / Yield (Two-Way)

Movem	ovement Performance - Vehicles Demand Deg Average Level of 95% Back of Queue Prop Effective Average													
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h			
South: J	ones S	Street												
2	Т	223	2.0	0.121	1.0	LOS A	0.8	5.9	0.38	0.00	45.5			
3	R	6	2.0	0.121	7.8	LOS A	0.8	5.9	0.38	0.88	43.0			
Approac	h	229	2.0	0.121	1.2	NA	0.8	5.9	0.38	0.02	45.5			
East: Oa	East: Oatlands Stre													
4	L	14	2.0	0.028	8.8	LOS A	0.1	0.7	0.38	0.59	41.2			
6	R	6	2.0	0.028	9.2	LOS A	0.1	0.7	0.38	0.73	41.1			
Approac	h	20	2.0	0.028	8.9	LOS A	0.1	0.7	0.38	0.63	41.2			
North: Jo	ones S	treet												
7	L	7	2.0	0.112	6.5	LOS A	0.0	0.0	0.00	0.91	43.3			
8	Т	207	2.0	0.112	0.0	LOS A	0.0	0.0	0.00	0.00	50.0			
Approac	h	215	2.0	0.112	0.2	NA	0.0	0.0	0.00	0.03	49.7			
All Vehic	cles	464	2.0	0.121	1.1	NA	0.8	5.9	0.20	0.05	47.1			

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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13S1210100 Jones Street-Oatlands Street Giveway / Yield (Two-Way)

Movem	vement Performance - Vehicles Demand Deg Average Level of 95% Back of Queue Prop Effective Average													
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h			
South: J	ones S	treet												
2	Т	232	2.0	0.135	1.5	LOS A	1.0	7.0	0.45	0.00	44.6			
3	R	16	2.0	0.135	8.3	LOS A	1.0	7.0	0.45	0.88	43.0			
Approac	h	247	2.0	0.135	1.9	NA	1.0	7.0	0.45	0.06	44.5			
East: Oatlands Stre		Street												
4	L	24	2.0	0.053	9.7	LOS A	0.2	1.3	0.45	0.65	40.5			
6	R	9	2.0	0.053	10.0	LOS A	0.2	1.3	0.45	0.79	40.4			
Approac	h	34	2.0	0.053	9.8	LOS A	0.2	1.3	0.45	0.69	40.5			
North: Jo	ones St	treet												
7	L	13	2.0	0.156	6.5	LOS A	0.0	0.0	0.00	0.90	43.3			
8	Т	286	2.0	0.156	0.0	LOS A	0.0	0.0	0.00	0.00	50.0			
Approac	h	299	2.0	0.156	0.3	NA	0.0	0.0	0.00	0.04	49.7			
All Vehic	cles	580	2.0	0.156	1.5	NA	1.0	7.0	0.22	0.08	46.8			

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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13S1210100 Jones Street-Oatlands Street Giveway / Yield (Two-Way)

Movem	ovement Performance - Vehicles													
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h			
South: J	lones S	Street												
2	Т	155	2.0	0.097	0.9	LOS A	0.6	4.4	0.35	0.00	45.6			
3	R	20	2.0	0.097	7.7	LOS A	0.6	4.4	0.35	0.85	42.9			
Approac	h	175	2.0	0.097	1.7	NA	0.6	4.4	0.35	0.10	45.3			
East: Oatland Stre		Street												
4	L	14	2.0	0.025	8.3	LOS A	0.1	0.6	0.36	0.59	41.6			
6	R	5	2.0	0.025	8.7	LOS A	0.1	0.6	0.36	0.71	41.4			
Approac	h	19	2.0	0.025	8.4	LOS A	0.1	0.6	0.36	0.62	41.6			
North: J	ones S	treet												
7	L	8	2.0	0.109	6.5	LOS A	0.0	0.0	0.00	0.90	43.3			
8	Т	201	2.0	0.109	0.0	LOS A	0.0	0.0	0.00	0.00	50.0			
Approac	h	209	2.0	0.109	0.3	NA	0.0	0.0	0.00	0.04	49.7			
All Vehic	cles	403	2.0	0.109	1.3	NA	0.6	4.4	0.17	0.09	47.3			

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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13S1210100 Jones Street-Smith Street Roundabout

Movem	Movement Performance - Vehicles Demand Deg. Average Level of 95% Back of Queue Prop. Effective Average													
	_	Demand	1.0.7	Deg.	Average	Level of	95% Back o	of Queue	Prop.	Effective	Average			
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed			
0 11 1	01	veh/h	%	v/c	sec		veh	m		per veh	km/h			
South: J	ones Str	eet -						. .						
1	L	5	2.0	0.225	6.7	LOS A	1.3	9.1	0.33	0.58	42.6			
2	Т	148	2.0	0.225	5.7	LOS A	1.3	9.1	0.33	0.49	43.0			
3	R	89	2.0	0.225	9.7	LOS A	1.3	9.1	0.33	0.73	40.8			
Approac	h	243	2.0	0.225	7.2	LOS A	1.3	9.1	0.33	0.58	42.2			
East: Sn	nith Stree	et												
4	L	25	2.0	0.113	7.0	LOS A	0.6	4.3	0.38	0.59	42.4			
5	Т	49	2.0	0.113	6.0	LOS A	0.6	4.3	0.38	0.51	42.7			
6	R	35	2.0	0.113	10.0	LOS A	0.6	4.3	0.38	0.72	40.7			
Approac	h	109	2.0	0.113	7.5	LOS A	0.6	4.3	0.38	0.60	42.0			
North: Jo	ones Stre	eet												
7	L	139	2.0	0.357	8.9	LOS A	2.2	15.6	0.63	0.74	41.3			
8	Т	119	2.0	0.357	7.9	LOS A	2.2	15.6	0.63	0.70	41.6			
9	R	29	2.0	0.357	12.0	LOS A	2.2	15.6	0.63	0.83	39.5			
Approac	h	287	2.0	0.357	8.8	LOS A	2.2	15.6	0.63	0.73	41.2			
West: Sr	mith Stre	et												
10	L	35	2.0	0.343	8.2	LOS A	2.1	14.8	0.55	0.70	42.1			
11	Т	253	2.0	0.343	7.2	LOS A	2.1	14.8	0.55	0.64	42.1			
12	R	15	2.0	0.343	11.2	LOS A	2.1	14.8	0.55	0.82	40.1			
Approac	h	302	2.0	0.343	7.5	LOS A	2.1	14.8	0.55	0.66	42.0			
All Vehic	les	942	2.0	0.357	7.8	LOS A	2.2	15.6	0.50	0.65	41.8			

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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13S1210100 Jones Street-Smith Street Roundabout

Movem	Movement Performance - Vehicles Demand Deg. Average Level of 95% Back of Queue Prop. Effective Average													
	-	Demand	1.0.7	Deg.	Average	Level of	95% Back of	of Queue	Prop.	Effective	Average			
Mov ID	lurn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed			
	01	veh/h	%	v/c	sec		veh	m		per veh	km/h			
South: J	ones Str	eet												
1	L	29	2.0	0.255	8.2	LOS A	1.4	10.3	0.55	0.70	42.0			
2	Т	157	2.0	0.255	7.3	LOS A	1.4	10.3	0.55	0.64	42.1			
3	R	29	2.0	0.255	11.3	LOS A	1.4	10.3	0.55	0.81	40.0			
Approac	h	216	2.0	0.255	8.0	LOS A	1.4	10.3	0.55	0.67	41.8			
East: Sn	nith Stree	et												
4	L	58	2.0	0.318	7.0	LOS A	2.0	14.1	0.42	0.61	42.4			
5	Т	208	2.0	0.318	6.1	LOS A	2.0	14.1	0.42	0.53	42.6			
6	R	64	2.0	0.318	10.1	LOS A	2.0	14.1	0.42	0.75	40.7			
Approac	h	331	2.0	0.318	7.0	LOS A	2.0	14.1	0.42	0.59	42.2			
North: Jo	ones Stre	eet												
7	L	122	2.0	0.252	6.9	LOS A	1.5	10.5	0.38	0.59	42.4			
8	Т	104	2.0	0.252	5.9	LOS A	1.5	10.5	0.38	0.51	42.8			
9	R	35	2.0	0.252	10.0	LOS A	1.5	10.5	0.38	0.73	40.8			
Approac	:h	261	2.0	0.252	6.9	LOS A	1.5	10.5	0.38	0.58	42.3			
West: Si	mith Stre	et												
10	L	23	2.0	0.150	7.6	LOS A	0.8	5.6	0.47	0.65	42.3			
11	Т	99	2.0	0.150	6.7	LOS A	0.8	5.6	0.47	0.58	42.5			
12	R	12	2.0	0.150	10.7	LOS A	0.8	5.6	0.47	0.78	40.4			
Approac	:h	134	2.0	0.150	7.2	LOS A	0.8	5.6	0.47	0.61	42.3			
All Vehic	cles	941	2.0	0.318	7.2	LOS A	2.0	14.1	0.44	0.61	42.1			

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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13S1210100 Jones Street-Smith Street Roundabout

Movem	Movement Performance - Vehicles Demand Deg. Average Level of 95% Back of Queue Prop. Effective Average													
		Demand		Deg.	Average	Level of	95% Back o	of Queue	Prop.	Effective	Average			
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed			
0	01	veh/h	%	v/c	sec		veh	m		per veh	km/h			
South: J	ones Str	eet												
1	L	20	2.0	0.161	6.9	LOS A	0.9	6.1	0.35	0.60	42.6			
2	Т	107	2.0	0.161	5.9	LOS A	0.9	6.1	0.35	0.51	43.0			
3	R	34	2.0	0.161	9.9	LOS A	0.9	6.1	0.35	0.75	40.8			
Approac	:h	161	2.0	0.161	6.9	LOS A	0.9	6.1	0.35	0.57	42.4			
East: Sn	nith Stree	et												
4	L	18	2.0	0.140	6.8	LOS A	0.7	5.2	0.34	0.59	42.6			
5	Т	83	2.0	0.140	5.9	LOS A	0.7	5.2	0.34	0.50	43.0			
6	R	40	2.0	0.140	9.9	LOS A	0.7	5.2	0.34	0.74	40.8			
Approac	:h	141	2.0	0.140	7.1	LOS A	0.7	5.2	0.34	0.58	42.3			
North: J	ones Str	eet												
7	L	88	2.0	0.207	6.9	LOS A	1.1	8.2	0.37	0.60	42.5			
8	Т	102	2.0	0.207	6.0	LOS A	1.1	8.2	0.37	0.52	42.8			
9	R	19	2.0	0.207	10.0	LOS A	1.1	8.2	0.37	0.75	40.8			
Approac	h	209	2.0	0.207	6.8	LOS A	1.1	8.2	0.37	0.57	42.5			
West: S	mith Stre	et												
10	L	11	2.0	0.129	7.1	LOS A	0.7	4.7	0.39	0.61	42.5			
11	т	99	2.0	0.129	6.1	LOS A	0.7	4.7	0.39	0.53	42.9			
12	R	15	2.0	0.129	10.2	LOS A	0.7	4.7	0.39	0.77	40.7			
Approac	:h	124	2.0	0.129	6.7	LOS A	0.7	4.7	0.39	0.57	42.6			
All Vehic	cles	636	2.0	0.207	6.9	LOS A	1.1	8.2	0.36	0.57	42.4			

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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13S1210100 Jones Street-Dunmore Street Roundabout

Movement Performanc			- Vehicles								
Mov ID	Turn	Demand Flow	HV	Deg. Satn	Average Delay	Level of Service	95% Back of Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
South:	Iones Str	ven/n	%	V/C	sec	_	ven	m	_	per ven	KM/N
1		158	2.0	0 311	8.8		1 9	13.4	0.62	0.73	41 2
2	Т	15	2.0	0.311	7.8	LOSA	1.5	13.4	0.02	0.75	41.2
3	R	75	2.0	0.311	11.8	LOSA	1.0	13.4	0.62	0.80	39.4
Approa	ch	247	2.0	0.311	9.6	LOSA	1.9	13.4	0.62	0.75	40.6
East: D	unmore S	Street									
4	L	54	2.0	0.426	8.4	LOS A	2.9	20.3	0.61	0.72	41.9
5	Т	312	2.0	0.426	7.4	LOS A	2.9	20.3	0.61	0.67	41.9
6	R	11	2.0	0.426	11.4	LOS A	2.9	20.3	0.61	0.82	40.0
Approa	ch	376	2.0	0.426	7.7	LOS A	2.9	20.3	0.61	0.68	41.8
North:	lones Stre	eet									
7	L	5	2.0	0.175	14.2	LOS A	1.0	7.5	0.84	0.89	37.3
8	Т	35	2.0	0.175	13.3	LOS A	1.0	7.5	0.84	0.87	37.5
9	R	35	2.0	0.175	17.3	LOS B	1.0	7.5	0.84	0.93	35.9
Approa	ch	75	2.0	0.175	15.2	LOS B	1.0	7.5	0.84	0.90	36.7
West: D	ounmore \$	Street									
10	L	21	2.0	0.704	7.2	LOS A	8.4	59.7	0.60	0.58	41.8
11	Т	628	2.0	0.704	6.2	LOS A	8.4	59.7	0.60	0.53	41.8
12	R	204	2.0	0.704	10.2	LOS A	8.4	59.7	0.60	0.68	40.6
Approa	ch	854	2.0	0.704	7.2	LOS A	8.4	59.7	0.60	0.56	41.5
All Vehi	cles	1552	2.0	0.704	8.1	LOS A	8.4	59.7	0.61	0.64	41.2

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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13S1210100 Jones Street-Dunmore Street Roundabout

Moven	nent Per	formance ·	- Vehicles								
	-	Demand	1.15.7	Deg.	Average	Level of	95% Back o	of Queue	Prop.	Effective	Average
MOV ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
Osutha	lanaa Ota	veh/h	%	V/C	sec		veh	m		per veh	km/h
South:	Jones Str	eet									
1	L	174	2.0	0.427	11.7	LOS A	2.9	20.9	0.81	0.91	39.0
2	Т	53	2.0	0.427	10.8	LOS A	2.9	20.9	0.81	0.89	39.2
3	R	35	2.0	0.427	14.8	LOS B	2.9	20.9	0.81	0.95	37.5
Approa	ch	261	2.0	0.427	11.9	LOS A	2.9	20.9	0.81	0.91	38.8
East: D	unmore S	Street									
4	L	61	2.0	0.634	9.4	LOS A	6.0	42.8	0.71	0.76	41.2
5	Т	525	2.0	0.634	8.4	LOS A	6.0	42.8	0.71	0.72	41.5
6	R	22	2.0	0.634	12.4	LOS A	6.0	42.8	0.71	0.84	39.4
Approach 6		608	2.0	0.634	8.7	LOS A	6.0	42.8	0.71	0.73	41.4
North:	lones Stre	eet									
7	L	12	2.0	0.102	8.9	LOS A	0.5	3.8	0.58	0.69	41.0
8	Т	18	2.0	0.102	8.0	LOS A	0.5	3.8	0.58	0.64	41.4
9	R	46	2.0	0.102	12.0	LOS A	0.5	3.8	0.58	0.77	39.2
Approa	ch	76	2.0	0.102	10.6	LOS A	0.5	3.8	0.58	0.73	40.0
West: D	ounmore \$	Street									
10	L	56	2.0	0.415	6.8	LOS A	3.1	22.2	0.41	0.58	42.3
11	т	233	2.0	0.415	5.8	LOS A	3.1	22.2	0.41	0.50	42.6
12	R	182	20	0 415	9.8	LOSA	31	22.2	0.41	0 71	40 7
Approa	ch .	.0 <u>2</u> 471	2.0	0.415	7.5		3.1	22.2	0.41	0.50	41.8
лрргоа		471	2.0	0.415	7.5	LUGA	5.1	22.2	0.41	0.59	41.0
All Vehi	cles	1416	2.0	0.634	9.0	LOS A	6.0	42.8	0.62	0.72	40.9

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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13S1210100 Jones Street-Dunmore Street Roundabout

Movem	ient Per	formance	- Vehicles								
		Demand	1.0.7	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
Coutbu	lamaa Ctr	veh/h	%	V/C	sec		veh	m		per veh	km/h
South: J	ones Str	eel	0.0	0.400	0.7	100.4		7.0	0.50	0.74	
1	L _	113	2.0	0.199	8.7	LOSA	1.1	7.8	0.58	0.71	41.4
2	I	19	2.0	0.199	1.1	LOSA	1.1	7.8	0.58	0.65	41./
3	R	24	2.0	0.199	11.7	LOS A	1.1	7.8	0.58	0.79	39.5
Approac	h	156	2.0	0.199	9.0	LOS A	1.1	7.8	0.58	0.71	41.1
East: Du	unmore S	Street									
4	L	28	2.0	0.385	7.6	LOS A	2.4	17.4	0.50	0.66	42.2
5	Т	333	2.0	0.385	6.7	LOS A	2.4	17.4	0.50	0.60	42.4
6	R	15	2.0	0.385	10.7	LOS A	2.4	17.4	0.50	0.80	40.5
Approach		376	2.0	0.385	6.9	LOS A	2.4	17.4	0.50	0.61	42.3
North: J	ones Stre	eet									
7	L	16	2.0	0.094	8.9	LOS A	0.5	3.4	0.56	0.69	41.1
8	Т	21	2.0	0.094	7.9	LOS A	0.5	3.4	0.56	0.63	41.5
9	R	34	2.0	0.094	12.0	LOS A	0.5	3.4	0.56	0.78	39.3
Approac	h	71	2.0	0.094	10.1	LOS A	0.5	3.4	0.56	0.71	40.3
West: D	unmore \$	Street									
10	L	34	2.0	0.362	6.3	LOS A	2.5	18.1	0.26	0.55	42.9
11	т	272	2.0	0.362	5.3	LOS A	2.5	18.1	0.26	0.46	43.4
12	R	152	2.0	0.362	9.4	LOS A	2.5	18.1	0.26	0.72	40.9
Approac	h	457	2.0	0.362	6.7	LOS A	2.5	18.1	0.26	0.55	42.5
All Vehic	cles	1059	2.0	0.385	7.4	LOS A	2.5	18.1	0.41	0.61	42.1

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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13S1210100

Dunmore Street-Goodall Street

Signals - Fixed Time Cycle Time = 65 seconds (Optimum Cycle Time - Minimum Delay)

Movem	Movement Performance - Vehicles											
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h	
East: Du	Inmore	Street										
5	Т	268	2.0	0.305	8.1	LOS A	4.4	31.4	0.55	0.47	40.5	
6	R	268	2.0	0.888	44.2	LOS D	9.4	67.2	1.00	1.16	24.2	
Approac	h	537	2.0	0.888	26.2	LOS B	9.4	67.2	0.78	0.81	30.3	
North: Goodall Stre		Street										
7	L	268	2.0	0.562	19.1	LOS B	5.6	39.7	0.69	0.77	34.2	
9	R	408	2.0	0.853	39.1	LOS C	14.9	105.9	1.00	1.01	25.8	
Approac	h	677	2.0	0.853	31.2	LOS C	14.9	105.9	0.88	0.91	28.6	
West: Du	unmore	Street										
10	L	333	2.0	0.308	9.7	LOS A	3.5	25.2	0.36	0.70	40.5	
11	Т	618	2.0	0.869	29.7	LOS C	22.6	161.1	1.00	1.09	27.9	
Approac	h	951	2.0	0.869	22.7	LOS B	22.6	161.1	0.77	0.96	31.3	
All Vehic	les	2164	2.0	0.888	26.2	LOS B	22.6	161.1	0.81	0.91	30.2	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians											
MoviD	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective			
	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate			
		ped/h	sec		ped	m		per ped			
P5	Across N approach	53	20.0	LOS C	0.1	0.1	0.78	0.78			
P7	Across W approach	53	25.9	LOS C	0.1	0.1	0.89	0.89			
All Ped	estrians	106	22.9	LOS C			0.84	0.84			

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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13S1210100

Dunmore Street-Goodall Street

Signals - Fixed Time Cycle Time = 56 seconds (Optimum Cycle Time - Minimum Delay)

Movem	Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h		
East: Du	inmore	Street											
5	Т	447	2.0	0.502	9.9	LOS A	8.1	57.4	0.70	0.60	38.7		
6	R	260	2.0	0.573	21.1	LOS B	5.7	40.4	0.91	0.82	33.1		
Approac	h	707	2.0	0.573	14.0	LOS A	8.1	57.4	0.77	0.68	36.5		
North: G	oodall \$	Street											
7	L	243	2.0	0.425	15.2	LOS B	3.9	27.5	0.61	0.75	36.6		
9	R	398	2.0	0.761	29.4	LOS C	11.1	79.3	0.97	0.92	29.3		
Approac	:h	641	2.0	0.761	24.0	LOS B	11.1	79.3	0.83	0.86	31.7		
West: D	unmore	Street											
10	L	233	2.0	0.216	10.0	LOS A	2.3	16.6	0.39	0.70	40.3		
11	Т	233	2.0	0.423	18.0	LOS B	5.3	37.9	0.86	0.71	33.4		
Approac	h	465	2.0	0.423	14.0	LOS A	5.3	37.9	0.62	0.71	36.5		
All Vehic	les	1814	2.0	0.761	17.6	LOS B	11.1	79.3	0.76	0.75	34.6		

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians											
MoviD	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective			
	Description	FIOW	Delay	Service	Pedestrian	Distance	Queued	Stop Rate			
		ped/h	sec		ped	m		per ped			
P5	Across N approach	53	22.3	LOS C	0.1	0.1	0.89	0.89			
P7	Across W approach	53	22.3	LOS C	0.1	0.1	0.89	0.89			
All Ped	estrians	106	22.3	LOS C			0.89	0.89			

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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13S1210100

Dunmore Street-Goodall Street

Signals - Fixed Time Cycle Time = 56 seconds (Optimum Cycle Time - Minimum Delay)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veb/b	HV %	Deg. Satn	Average Delay	Level of Service	95% Back o Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed km/b	
East: Du	inmore \$	Street	70	V/C	300		VCIT				KIII/II	
5	Т	281	2.0	0.315	8.9	LOS A	4.5	32.3	0.62	0.53	39.7	
6	R	201	2.0	0.451	21.0	LOS B	4.2	30.1	0.87	0.79	33.2	
Approac	h	482	2.0	0.451	13.9	LOS A	4.5	32.3	0.73	0.64	36.7	
North: Goodall Stre		Street										
7	L	195	2.0	0.340	15.0	LOS B	3.0	21.4	0.59	0.74	36.8	
9	R	209	2.0	0.400	24.4	LOS B	4.8	34.0	0.85	0.79	31.5	
Approac	h	404	2.0	0.400	19.9	LOS B	4.8	34.0	0.73	0.77	33.8	
West: Du	unmore	Street										
10	L	165	2.0	0.157	9.9	LOS A	1.6	11.6	0.37	0.69	40.4	
11	Т	269	2.0	0.482	18.2	LOS B	6.2	44.2	0.87	0.72	33.3	
Approac	h	435	2.0	0.482	15.0	LOS B	6.2	44.2	0.68	0.71	35.7	
All Vehic	les	1321	2.0	0.482	16.1	LOS B	6.2	44.2	0.71	0.70	35.4	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians											
Mov ID	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective			
	Description	ped/h	sec	Service	pedestnan	Distance	Queued	ber bed			
P5	Across N approach	53	22.3	LOS C	0.1	0.1	0.89	0.89			
P7	Across W approach	53	22.3	LOS C	0.1	0.1	0.89	0.89			
All Ped	estrians	106	22.3	LOS C			0.89	0.89			

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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13S1210100

Dunmore Street-Pendle Way Signals - Fixed Time Cycle Time = 60 seconds (Optimum Cycle Time - Minimum Delay)

Movem	Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow	HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed		
South: E	Pondlo V	veh/h	%	V/C	sec		veh	m		per veh	km/h		
South. F		vay											
2	Т	236	2.0	0.204	5.9	LOS A	3.1	22.4	0.48	0.41	42.5		
3	R	516	2.0	0.780	25.8	LOS B	12.7	90.1	0.95	1.03	30.8		
Approac	:h	752	2.0	0.780	19.5	LOS B	12.7	90.1	0.80	0.83	33.7		
East: Du	Inmore	Street											
4	L	306	2.0	0.314	15.0	LOS B	5.1	36.0	0.60	0.76	36.8		
6	R	263	2.0	0.719	33.4	LOS C	7.9	56.3	0.99	0.89	27.7		
Approac	:h	569	2.0	0.719	23.5	LOS B	7.9	56.3	0.78	0.82	31.9		
North: P	endle V	Vay											
7	L	408	2.0	0.450	14.3	LOS A	6.7	47.8	0.60	0.76	37.2		
8	Т	188	2.0	0.367	19.8	LOS B	4.6	32.9	0.86	0.70	32.4		
Approac	:h	597	2.0	0.450	16.0	LOS B	6.7	47.8	0.68	0.74	35.6		
All Vehic	les	1918	2.0	0.780	19.6	LOS B	12.7	90.1	0.76	0.80	33.7		

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians											
MoviD	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective			
	Description	FIOW	Delay	Service	Pedestrian	Distance	Queuea	Stop Rate			
		ped/h	sec		ped	m		per ped			
P1	Across S approach	53	12.0	LOS B	0.1	0.1	0.63	0.63			
P3	Across E approach	53	24.3	LOS C	0.1	0.1	0.90	0.90			
All Pede	estrians	106	18.2	LOS B			0.77	0.77			

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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13S1210100

Dunmore Street-Pendle Way

Signals - Fixed Time Cycle Time = 60 seconds (Optimum Cycle Time - Minimum Delay)

Movem	Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h		
South: P	Pendle V	Way											
2	Т	277	2.0	0.308	10.8	LOS A	5.1	36.2	0.66	0.56	38.2		
3	R	255	2.0	0.573	22.5	LOS B	6.0	42.7	0.91	0.82	32.4		
Approac	h	532	2.0	0.573	16.4	LOS B	6.0	42.7	0.78	0.68	35.2		
East: Du	inmore	Street											
4	L	420	2.0	0.430	15.7	LOS B	7.5	53.5	0.65	0.78	36.3		
6	R	398	2.0	0.652	25.5	LOS B	10.3	73.2	0.91	0.84	31.0		
Approac	h	818	2.0	0.652	20.5	LOS B	10.3	73.2	0.77	0.81	33.5		
North: P	endle V	Vay											
7	L	199	2.0	0.184	9.7	LOS A	1.9	13.9	0.36	0.69	40.6		
8	Т	188	2.0	0.367	19.8	LOS B	4.6	32.9	0.86	0.70	32.4		
Approac	h	387	2.0	0.367	14.6	LOS B	4.6	32.9	0.60	0.70	36.2		
All Vehic	les	1737	2.0	0.652	17.9	LOS B	10.3	73.2	0.74	0.75	34.6		

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians											
MaxID	Description	Demand	Average	Average Level of A		of Queue	Prop.	Effective			
	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate			
		ped/h	sec		ped	m		per ped			
P1	Across S approach	53	12.0	LOS B	0.1	0.1	0.63	0.63			
P3	Across E approach	53	24.3	LOS C	0.1	0.1	0.90	0.90			
All Pede	estrians	106	18.2	LOS B			0.77	0.77			

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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13S1210100

Dunmore Street-Pendle Way

Signals - Fixed Time Cycle Time = 50 seconds (Optimum Cycle Time - Minimum Delay)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h	
South: P	Pendle V	Vay										
2	Т	303	2.0	0.449	6.5	LOS A	4.2	29.6	0.57	0.48	41.5	
3	R	217	2.0	0.449	16.0	LOS B	4.2	29.6	0.79	0.80	36.2	
Approac	h	520	2.0	0.449	10.4	LOS A	4.2	29.6	0.66	0.61	39.1	
East: Du	inmore	Street										
4	L	255	2.0	0.316	16.5	LOS B	4.1	29.4	0.69	0.77	35.8	
6	R	238	2.0	0.650	28.2	LOS B	5.8	41.4	0.97	0.86	29.8	
Approac	h	493	2.0	0.650	22.1	LOS B	5.8	41.4	0.82	0.81	32.6	
North: P	endle W	Vay										
7	L	226	2.0	0.211	10.4	LOS A	2.3	16.2	0.44	0.71	40.0	
8	Т	222	2.0	0.361	14.5	LOS B	4.3	30.6	0.81	0.67	35.5	
Approac	h	448	2.0	0.361	12.5	LOS A	4.3	30.6	0.62	0.69	37.6	
All Vehic	les	1461	2.0	0.650	15.0	LOS B	5.8	41.4	0.70	0.70	36.2	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	Movement Performance - Pedestrians													
	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective						
	Description	FIOW	Delay	Service	Pedestrian	Distance	Queued	Stop Rate						
		pea/n	sec		ped	m		per ped						
P1	Across S approach	53	14.4	LOS B	0.1	0.1	0.76	0.76						
P3	Across E approach	53	19.4	LOS B	0.1	0.1	0.88	0.88						
All Pede	estrians	106	16.9	LOS B			0.82	0.82						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.



Movem	Movement Performance - Vehicles													
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h			
South: J	lones St	reet												
2	Т	274	2.0	0.147	1.6	LOS A	1.1	8.0	0.47	0.00	44.5			
3	R	5	2.0	0.147	8.4	LOS A	1.1	8.0	0.47	0.90	43.1			
Approac	ch	279	2.0	0.147	1.7	NA	1.1	8.0	0.47	0.02	44.5			
East: Ro	ogers St	reet												
4	L	5	2.0	0.022	11.3	LOS A	0.1	0.5	0.51	0.62	39.3			
6	R	5	2.0	0.022	11.6	LOS A	0.1	0.5	0.51	0.78	39.1			
Approac	h	11	2.0	0.022	11.4	LOS A	0.1	0.5	0.51	0.70	39.2			
North: J	ones St	reet												
7	L	5	2.0	0.160	6.5	LOS A	0.0	0.0	0.00	0.92	43.3			
8	Т	302	2.0	0.160	0.0	LOS A	0.0	0.0	0.00	0.00	50.0			
Approac	h	307	2.0	0.160	0.1	NA	0.0	0.0	0.00	0.02	49.9			
All Vehic	cles	597	2.0	0.160	1.0	NA	1.1	8.0	0.23	0.03	47.0			

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Movement Performance - Vehicles													
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h		
South: J	ones St	reet											
2	Т	287	2.0	0.161	1.6	LOS A	1.2	8.8	0.48	0.00	44.4		
3	R	13	2.0	0.161	8.4	LOS A	1.2	8.8	0.48	0.89	43.0		
Approac	h	300	2.0	0.161	1.9	NA	1.2	8.8	0.48	0.04	44.4		
East: Ro	ogers St	reet											
4	L	13	2.0	0.034	10.4	LOS A	0.1	0.8	0.48	0.64	39.9		
6	R	6	2.0	0.034	10.8	LOS A	0.1	0.8	0.48	0.80	39.8		
Approac	h	19	2.0	0.034	10.5	LOS A	0.1	0.8	0.48	0.69	39.9		
North: J	ones St	reet											
7	L	6	2.0	0.160	6.5	LOS A	0.0	0.0	0.00	0.91	43.3		
8	Т	301	2.0	0.160	0.0	LOS A	0.0	0.0	0.00	0.00	50.0		
Approac	h	307	2.0	0.160	0.1	NA	0.0	0.0	0.00	0.02	49.8		
All Vehic	cles	626	2.0	0.161	1.3	NA	1.2	8.8	0.24	0.05	46.7		

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Movement Performance - Vehicles													
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h		
South: J	ones S	Street											
2	Т	183	2.0	0.101	1.1	LOS A	0.7	4.9	0.39	0.00	45.3		
3	R	6	2.0	0.101	7.9	LOS A	0.7	4.9	0.39	0.88	43.0		
Approac	h	189	2.0	0.101	1.3	NA	0.7	4.9	0.39	0.03	45.3		
East: Ro	ogers S	street											
4	L	13	2.0	0.029	9.0	LOS A	0.1	0.7	0.40	0.60	41.1		
6	R	7	2.0	0.029	9.3	LOS A	0.1	0.7	0.40	0.73	40.9		
Approac	h	20	2.0	0.029	9.1	LOS A	0.1	0.7	0.40	0.65	41.0		
North: J	ones S	treet											
7	L	16	2.0	0.125	6.5	LOS A	0.0	0.0	0.00	0.89	43.3		
8	Т	223	2.0	0.125	0.0	LOS A	0.0	0.0	0.00	0.00	50.0		
Approac	h	239	2.0	0.125	0.4	NA	0.0	0.0	0.00	0.06	49.5		
All Vehic	cles	448	2.0	0.125	1.2	NA	0.7	4.9	0.18	0.07	47.2		

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Movem	Movement Performance - Vehicles													
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h			
South: J	lones Str	reet												
2	Т	253	2.0	0.137	1.1	LOS A	1.0	6.9	0.41	0.00	45.2			
3	R	6	2.0	0.137	7.9	LOS A	1.0	6.9	0.41	0.88	43.0			
Approac	ch	259	2.0	0.137	1.3	NA	1.0	6.9	0.41	0.02	45.1			
East: Oa	atlands S	Street												
4	L	16	2.0	0.033	9.1	LOS A	0.1	0.8	0.41	0.61	41.0			
6	R	6	2.0	0.033	9.5	LOS A	0.1	0.8	0.41	0.75	40.8			
Approac	ch	22	2.0	0.033	9.2	LOS A	0.1	0.8	0.41	0.65	40.9			
North: J	ones Str	eet												
7	L	7	2.0	0.126	6.5	LOS A	0.0	0.0	0.00	0.91	43.3			
8	Т	235	2.0	0.126	0.0	LOS A	0.0	0.0	0.00	0.00	50.0			
Approac	ch	242	2.0	0.126	0.2	NA	0.0	0.0	0.00	0.03	49.8			
All Vehic	cles	523	2.0	0.137	1.1	NA	1.0	6.9	0.22	0.05	46.9			

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Movement Performance - Vehicles													
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h		
South: J	ones S	treet											
2	Т	262	2.0	0.154	1.8	LOS A	1.2	8.4	0.49	0.00	44.2		
3	R	18	2.0	0.154	8.6	LOS A	1.2	8.4	0.49	0.89	42.9		
Approac	h	280	2.0	0.154	2.2	NA	1.2	8.4	0.49	0.06	44.1		
East: Oa	atlands	Street											
4	L	27	2.0	0.065	10.3	LOS A	0.2	1.6	0.49	0.67	40.0		
6	R	11	2.0	0.065	10.7	LOS A	0.2	1.6	0.49	0.83	39.9		
Approac	h	38	2.0	0.065	10.4	LOS A	0.2	1.6	0.49	0.72	40.0		
North: J	ones St	reet											
7	L	15	2.0	0.176	6.5	LOS A	0.0	0.0	0.00	0.90	43.3		
8	Т	323	2.0	0.176	0.0	LOS A	0.0	0.0	0.00	0.00	50.0		
Approac	h	338	2.0	0.176	0.3	NA	0.0	0.0	0.00	0.04	49.7		
All Vehic	cles	656	2.0	0.176	1.7	NA	1.2	8.4	0.24	0.09	46.5		

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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13S1210100 Jones Street-Oatlands Street Giveway / Yield (Two-Way)

Movement Performance - Vehicles													
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h		
South: J	ones S	street											
2	Т	175	2.0	0.110	1.1	LOS A	0.7	5.1	0.38	0.00	45.3		
3	R	22	2.0	0.110	7.9	LOS A	0.7	5.1	0.38	0.85	42.9		
Approac	h	197	2.0	0.110	1.8	NA	0.7	5.1	0.38	0.10	45.0		
East: Oa	atland S	Street											
4	L	16	2.0	0.031	8.7	LOS A	0.1	0.8	0.39	0.60	41.3		
6	R	6	2.0	0.031	9.1	LOS A	0.1	0.8	0.39	0.73	41.1		
Approac	h	22	2.0	0.031	8.8	LOS A	0.1	0.8	0.39	0.64	41.3		
North: J	ones S	treet											
7	L	9	2.0	0.123	6.5	LOS A	0.0	0.0	0.00	0.90	43.3		
8	Т	227	2.0	0.123	0.0	LOS A	0.0	0.0	0.00	0.00	50.0		
Approac	h	237	2.0	0.123	0.3	NA	0.0	0.0	0.00	0.04	49.7		
All Vehic	cles	456	2.0	0.123	1.4	NA	0.7	5.1	0.18	0.09	47.1		

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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13S1210100 Jones Street-Smith Street Roundabout

Moven	nent Per	formance ·	- Vehicles								
Mov ID	Turn	Demand Flow	HV ∞	Deg. Satn	Average Delay	Level of Service	95% Back o Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
South:	Jones Str	eet	70	V/C	Sec	_	ven		_	perven	K111/11
1	L	5	2.0	0.257	6.8	LOS A	1.5	10.7	0.36	0.59	42.5
2	Т	167	2.0	0.257	5.9	LOSA	1.5	10.7	0.36	0.51	42.9
3	R	101	2.0	0.257	9.9	LOS A	1.5	10.7	0.36	0.73	40.8
Approa	ch	274	2.0	0.257	7.4	LOS A	1.5	10.7	0.36	0.59	42.1
East: S	mith Stree	et									
4	L	28	2.0	0.130	7.1	LOS A	0.7	5.0	0.41	0.60	42.3
5	Т	56	2.0	0.130	6.2	LOS A	0.7	5.0	0.41	0.53	42.6
6	R	39	2.0	0.130	10.2	LOS A	0.7	5.0	0.41	0.73	40.6
Approa	ch	123	2.0	0.130	7.7	LOS A	0.7	5.0	0.41	0.61	41.9
North: J	lones Stre	eet									
7	L	157	2.0	0.422	9.5	LOS A	2.7	19.4	0.69	0.79	40.8
8	Т	134	2.0	0.422	8.5	LOS A	2.7	19.4	0.69	0.75	41.1
9	R	34	2.0	0.422	12.6	LOS A	2.7	19.4	0.69	0.86	39.1
Approa	ch	324	2.0	0.422	9.4	LOS A	2.7	19.4	0.69	0.78	40.8
West: S	mith Stre	et									
10	L	39	2.0	0.400	8.6	LOS A	2.5	18.1	0.61	0.73	41.8
11	Т	285	2.0	0.400	7.7	LOS A	2.5	18.1	0.61	0.68	41.9
12	R	17	2.0	0.400	11.7	LOS A	2.5	18.1	0.61	0.84	39.8
Approa	ch	341	2.0	0.400	8.0	LOS A	2.5	18.1	0.61	0.69	41.8
All Vehi	cles	1062	2.0	0.422	8.2	LOS A	2.7	19.4	0.55	0.68	41.5

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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13S1210100 Jones Street-Smith Street Roundabout

Movem	Movement Performance - Vehicles													
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h			
South: J	lones Str	eet												
1	L	33	2.0	0.297	8.6	LOS A	1.7	12.5	0.59	0.73	41.6			
2	Т	177	2.0	0.297	7.7	LOS A	1.7	12.5	0.59	0.68	41.9			
3	R	33	2.0	0.297	11.7	LOS A	1.7	12.5	0.59	0.83	39.7			
Approad	ch	242	2.0	0.297	8.4	LOS A	1.7	12.5	0.59	0.70	41.5			
East: Sr		el 05	2.0	0.205	7.0		2.4	17.0	0.40	0.02	40.0			
4	L -	60	2.0	0.305	7.3	LOSA	2.4	17.0	0.40	0.63	42.2			
5	I	236	2.0	0.365	6.3	LOSA	2.4	17.0	0.46	0.56	42.4			
6	<u>к</u>	72	2.0	0.365	10.3	LUSA	2.4	17.0	0.46	0.76	40.6			
Approac	n	373	2.0	0.365	7.3	LOSA	2.4	17.0	0.46	0.61	42.0			
North: J	ones Stre	eet												
7	L	138	2.0	0.289	7.1	LOS A	1.8	12.5	0.42	0.61	42.3			
8	Т	118	2.0	0.289	6.1	LOS A	1.8	12.5	0.42	0.53	42.6			
9	R	39	2.0	0.289	10.1	LOS A	1.8	12.5	0.42	0.74	40.7			
Approad	h	295	2.0	0.289	7.1	LOS A	1.8	12.5	0.42	0.59	42.2			
West: S	mith Stre	et												
10	L	26	2.0	0.174	7.9	LOS A	0.9	6.7	0.50	0.67	42.2			
11	Т	112	2.0	0.174	6.9	LOS A	0.9	6.7	0.50	0.60	42.3			
12	R	13	2.0	0.174	10.9	LOS A	0.9	6.7	0.50	0.79	40.2			
Approac	h	151	2.0	0.174	7.4	LOS A	0.9	6.7	0.50	0.63	42.1			
All Vehic	cles	1060	2.0	0.365	7.5	LOS A	2.4	17.0	0.49	0.63	42.0			

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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13S1210100 Jones Street-Smith Street Roundabout

Movem	Movement Performance - Vehicles Demand Deg. Average Level of 95% Back of Queue <u>Prop. Effective Average</u>													
	-	Demand		Deg.	Average	Level of	95% Back o	of Queue	Prop.	Effective	Average			
Mov ID	lurn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed			
	01	veh/h	%	v/c	sec		veh	m		per veh	km/h			
South: J	ones Str	eet												
1	L	22	2.0	0.184	7.0	LOS A	1.0	7.1	0.38	0.61	42.5			
2	Т	121	2.0	0.184	6.1	LOS A	1.0	7.1	0.38	0.53	42.8			
3	R	38	2.0	0.184	10.1	LOS A	1.0	7.1	0.38	0.75	40.7			
Approac	h	181	2.0	0.184	7.0	LOS A	1.0	7.1	0.38	0.58	42.3			
East: Sn	nith Stree	et												
4	L	20	2.0	0.161	7.0	LOS A	0.9	6.1	0.37	0.60	42.5			
5	Т	94	2.0	0.161	6.0	LOS A	0.9	6.1	0.37	0.51	42.8			
6	R	45	2.0	0.161	10.0	LOS A	0.9	6.1	0.37	0.74	40.8			
Approac	h	159	2.0	0.161	7.3	LOS A	0.9	6.1	0.37	0.59	42.2			
North: Jo	ones Stre	eet												
7	L	100	2.0	0.239	7.1	LOS A	1.4	9.7	0.41	0.61	42.4			
8	Т	116	2.0	0.239	6.2	LOS A	1.4	9.7	0.41	0.54	42.7			
9	R	21	2.0	0.239	10.2	LOS A	1.4	9.7	0.41	0.75	40.7			
Approac	:h	237	2.0	0.239	6.9	LOS A	1.4	9.7	0.41	0.59	42.4			
West: Si	mith Stre	et												
10	L	12	2.0	0.149	7.3	LOS A	0.8	5.5	0.42	0.63	42.5			
11	Т	112	2.0	0.149	6.3	LOS A	0.8	5.5	0.42	0.55	42.7			
12	R	17	2.0	0.149	10.4	LOS A	0.8	5.5	0.42	0.78	40.6			
Approac	:h	140	2.0	0.149	6.9	LOS A	0.8	5.5	0.42	0.58	42.4			
All Vehic	cles	717	2.0	0.239	7.0	LOS A	1.4	9.7	0.40	0.59	42.3			

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.



13S1210100 Jones Street-Dunmore Street Roundabout

Movem	Movement Performance - Vehicles													
MovuB	Turn	Demand	Ц\/	Deg.	Average	Level of	95% Back o	of Queue	Prop.	Effective	Average			
	Turri	Flow	□ V 0/	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed			
South:	ones Str	ven/n	%	V/C	Sec	_	ven	m	_	per ven	KM/N			
1	L	179	2.0	0.371	9.3	LOS A	2.4	16.8	0.68	0.77	40.8			
2	Т	17	2.0	0.371	8.3	LOSA	2.4	16.8	0.68	0.73	41.1			
3	R	84	2.0	0.371	12.4	LOS A	2.4	16.8	0.68	0.83	39.0			
Approac	h	280	2.0	0.371	10.2	LOS A	2.4	16.8	0.68	0.79	40.2			
East: Di	inmore S	Stroot												
4		61	20	0.504	9.0	LOSA	37	26.6	0.69	0 77	41.5			
5	T	352	2.0	0.504	8.0	LOSA	37	26.6	0.69	0.73	41.5			
6	R	13	2.0	0.504	12.1	LOSA	3.7	26.6	0.69	0.85	39.6			
Approac	:h	425	2.0	0.504	8.3	LOSA	3.7	26.6	0.69	0.73	41.5			
	<u>.</u>													
North: J	ones Stro	eet												
7	L	5	2.0	0.252	16.6	LOS B	1.6	11.5	0.92	0.97	35.7			
8	Т	39	2.0	0.252	15.7	LOS B	1.6	11.5	0.92	0.96	35.8			
9	R	39	2.0	0.252	19.7	LOS B	1.6	11.5	0.92	0.98	34.5			
Approac	h	83	2.0	0.252	17.6	LOS B	1.6	11.5	0.92	0.97	35.2			
West: D	unmore	Street												
10	L	24	2.0	0.808	7.8	LOS A	12.1	86.1	0.80	0.60	41.2			
11	Т	709	2.0	0.808	6.8	LOS A	12.1	86.1	0.80	0.57	40.9			
12	R	231	2.0	0.808	10.8	LOS A	12.1	86.1	0.80	0.65	40.4			
Approac	h	964	2.0	0.808	7.8	LOS A	12.1	86.1	0.80	0.59	40.8			
All Vehic	cles	1753	2.0	0.808	8.8	LOS A	12.1	86.1	0.76	0.67	40.6			

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

Processed: Wednesday, 19 March 2014 8:46:07 AM SIDRA INTERSECTION 5.1.13.2093 Copyright © 2000-2011 Akcelik and Associates Pty Ltd www.sidrasolutions.com Project: P:\13S1200-1299\13S1210100 Bonds Pendle Hill Amended Report\Modelling\140318sid - 13S1210100 Bonds Spinning Mills - 2027 sin



Bonds Spinning Mills - 2027.sip 8000056, GTA CONSULTANTS, ENTERPRISE

13S1210100 Jones Street-Dunmore Street Roundabout

Moven	nent Per	formance	- Vehicles								
Mov ID	Turn	Demand Flow	HV	Deg. Satn	Average Delay	Level of Service	95% Back of Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
South:	lones Str	ven/n	%	V/C	sec	_	ven	m	_	per ven	Km/n
1		106	20	0 543	15.0	LOSB	4.5	32.0	0 00	1.05	36.8
2	Т	59	2.0	0.543	14.0		4.5	32.0	0.30	1.05	36.0
3	R	39	2.0	0.543	14.0	LOSB	4.5	32.0	0.00	1.04	35.5
Approa	ch	294	2.0	0.543	15.2	LOS B	4.5	32.0	0.90	1.05	36.6
East: D	unmore S	street									
4	L	68	2.0	0.741	12.1	LOS A	9.5	67.6	0.85	0.89	39.2
5	Т	594	2.0	0.741	11.1	LOS A	9.5	67.6	0.85	0.87	39.3
6	R	25	2.0	0.741	15.1	LOS B	9.5	67.6	0.85	0.93	37.6
Approa	ch	687	2.0	0.741	11.4	LOS A	9.5	67.6	0.85	0.88	39.3
North: J	lones Stre	eet									
7	L	13	2.0	0.123	9.5	LOS A	0.7	4.7	0.63	0.72	40.6
8	Т	20	2.0	0.123	8.5	LOS A	0.7	4.7	0.63	0.68	41.0
9	R	53	2.0	0.123	12.5	LOS A	0.7	4.7	0.63	0.80	38.8
Approa	ch	85	2.0	0.123	11.1	LOS A	0.7	4.7	0.63	0.76	39.6
West: D	ounmore S	Street									
10	L	62	2.0	0.476	7.0	LOS A	3.9	27.6	0.47	0.59	42.1
11	Т	262	2.0	0.476	6.0	LOS A	3.9	27.6	0.47	0.52	42.3
12	R	206	2.0	0.476	10.0	LOS A	3.9	27.6	0.47	0.71	40.6
Approa	ch	531	2.0	0.476	7.7	LOS A	3.9	27.6	0.47	0.60	41.6
All Vehi	cles	1597	2.0	0.741	10.8	LOS A	9.5	67.6	0.72	0.81	39.5

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.



13S1210100 Jones Street-Dunmore Street Roundabout

Movem	ient Pei	formance	- Vehicles								
MovuD	Turn	Demand	Ш\/	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
	Turri	Flow	□ V 0/	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
South: J	lones Sti	reet	70	۷/۵	Sec	_	ven	111	_	perven	K111/11
1	L	127	2.0	0.237	9.1	LOS A	1.4	9.6	0.63	0.74	41.0
2	т	21	2.0	0.237	8.2	LOS A	1.4	9.6	0.63	0.70	41.3
3	R	27	2.0	0.237	12.2	LOS A	1.4	9.6	0.63	0.82	39.1
Approac	h	176	2.0	0.237	9.5	LOS A	1.4	9.6	0.63	0.75	40.7
East: Du	unmore S	Street									
4	L	33	2.0	0.447	8.0	LOS A	3.0	21.5	0.56	0.69	42.1
5	Т	376	2.0	0.447	7.1	LOS A	3.0	21.5	0.56	0.63	42.1
6	R	17	2.0	0.447	11.1	LOS A	3.0	21.5	0.56	0.81	40.3
Approad	h	425	2.0	0.447	7.3	LOS A	3.0	21.5	0.56	0.64	42.0
North: J	ones Str	eet									
7	L	18	2.0	0.112	9.4	LOS A	0.6	4.1	0.61	0.72	40.7
8	Т	24	2.0	0.112	8.5	LOS A	0.6	4.1	0.61	0.67	41.1
9	R	38	2.0	0.112	12.5	LOS A	0.6	4.1	0.61	0.80	38.9
Approad	h	80	2.0	0.112	10.6	LOS A	0.6	4.1	0.61	0.74	39.9
West: D	unmore	Street									
10	L	38	2.0	0.412	6.4	LOS A	3.1	22.1	0.30	0.55	42.7
11	Т	307	2.0	0.412	5.4	LOS A	3.1	22.1	0.30	0.46	43.2
12	R	172	2.0	0.412	9.4	LOS A	3.1	22.1	0.30	0.71	40.9
Approad	h	517	2.0	0.412	6.8	LOS A	3.1	22.1	0.30	0.55	42.3
All Vehic	cles	1198	2.0	0.447	7.6	LOS A	3.1	22.1	0.46	0.63	41.8

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.



Dunmore Street-Goodall Street

Signals - Fixed Time Cycle Time = 85 seconds (Optimum Cycle Time - Minimum Delay)

Movem	ent Pe	erformance - \	/ ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
East: Du	inmore	Street									
5	Т	303	2.0	0.381	9.1	LOS A	6.1	43.3	0.52	0.45	39.7
6	R	303	2.0	0.913	59.1	LOS E	14.1	100.3	1.00	1.23	20.6
Approac	h	606	2.0	0.913	34.1	LOS C	14.1	100.3	0.76	0.84	27.1
North: G	oodall :	Street									
7	L	303	2.0	0.745	26.3	LOS B	9.0	64.3	0.69	0.83	30.6
9	R	461	2.0	0.931	59.1	LOS E	24.9	177.3	1.00	1.09	20.6
Approac	h	764	2.0	0.931	46.1	LOS D	24.9	177.3	0.88	0.98	23.7
West: D	unmore	Street									
10	L	376	2.0	0.387	10.6	LOS A	5.2	37.2	0.36	0.71	39.8
11	Т	698	2.0	0.934	47.8	LOS D	37.8	269.4	1.00	1.19	22.4
Approac	h	1074	2.0	0.934	34.8	LOS C	37.8	269.4	0.78	1.02	26.5
All Vehic	les	2444	2.0	0.934	38.2	LOS C	37.8	269.4	0.80	0.96	25.7

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians													
Movin	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective					
	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate					
		ped/h	sec		ped	m		per ped					
P5	Across N approach	53	22.6	LOS C	0.1	0.1	0.73	0.73					
P7	Across W approach	53	30.5	LOS D	0.1	0.1	0.85	0.85					
All Ped	estrians	106	26.6	LOS C			0.79	0.79					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.



Dunmore Street-Goodall Street

Signals - Fixed Time Cycle Time = 60 seconds (Optimum Cycle Time - Minimum Delay)

Movem	ent Pe	erformance - V	Vehicles								
Mov ID	Turn	Demand Flow veh/ <u>h</u>	HV %	Deg. Satn v/c	Average Delay se <u>c</u>	Level of Service	95% Back o Vehicles veh	of Queue Distance <u>m</u>	Prop. Queued	Effective Stop Rate per veh	Average Speed km/ <u>h</u>
East: Du	inmore	Street									
5	Т	506	2.0	0.612	12.6	LOS A	10.9	77.4	0.77	0.68	36.7
6	R	294	2.0	0.740	28.3	LOS B	8.1	57.9	0.99	0.94	29.7
Approac	h	800	2.0	0.740	18.4	LOS B	10.9	77.4	0.85	0.78	33.8
North: G	oodall :	Street									
7	L	275	2.0	0.479	14.8	LOS B	4.4	31.6	0.58	0.74	36.9
9	R	449	2.0	0.736	27.8	LOS B	12.6	90.1	0.94	0.89	30.0
Approac	:h	724	2.0	0.736	22.8	LOS B	12.6	90.1	0.81	0.84	32.3
West: D	unmore	Street									
10	L	262	2.0	0.243	9.8	LOS A	2.7	19.0	0.37	0.70	40.5
11	Т	262	2.0	0.511	20.8	LOS B	6.7	47.9	0.90	0.75	31.9
Approac	h	524	2.0	0.511	15.3	LOS B	6.7	47.9	0.64	0.72	35.7
All Vehic	les	2048	2.0	0.740	19.2	LOS B	12.6	90.1	0.78	0.78	33.7

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians													
May ID	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective					
	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate					
		ped/h	sec		ped	m		per ped					
P5	Across N approach	53	24.3	LOS C	0.1	0.1	0.90	0.90					
P7	Across W approach	53	20.8	LOS C	0.1	0.1	0.83	0.83					
All Ped	estrians	106	22.6	LOS C			0.87	0.87					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.



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Dunmore Street-Goodall Street

Signals - Fixed Time Cycle Time = 56 seconds (Optimum Cycle Time - Minimum Delay)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h	
East: Du	inmore	Street								· ·		
5	Т	318	2.0	0.356	9.1	LOS A	5.3	37.4	0.64	0.54	39.5	
6	R	227	2.0	0.535	22.2	LOS B	5.0	35.9	0.91	0.81	32.5	
Approac	h	545	2.0	0.535	14.6	LOS B	5.3	37.4	0.75	0.65	36.2	
North: G	ioodall S	Street										
7	L	220	2.0	0.385	15.1	LOS B	3.4	24.5	0.60	0.74	36.7	
9	R	237	2.0	0.453	24.7	LOS B	5.5	39.1	0.87	0.80	31.3	
Approac	h	457	2.0	0.453	20.1	LOS B	5.5	39.1	0.74	0.77	33.7	
West: D	unmore	Street										
10	L	186	2.0	0.168	10.0	LOS A	2.1	14.9	0.38	0.71	40.4	
11	Т	304	2.0	0.514	17.6	LOS B	6.7	47.6	0.85	0.71	33.6	
Approac	h	491	2.0	0.514	14.7	LOS B	6.7	47.6	0.67	0.71	35.9	
All Vehic	les	1493	2.0	0.535	16.3	LOS B	6.7	47.6	0.72	0.71	35.3	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Mover	Movement Performance - Pedestrians												
		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective					
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate					
		ped/h	sec		ped	m		per ped					
P5	Across N approach	53	22.3	LOS C	0.1	0.1	0.89	0.89					
P7	Across W approach	53	22.3	LOS C	0.1	0.1	0.89	0.89					
All Ped	estrians	106	22.3	LOS C			0.89	0.89					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.



Dunmore Street-Pendle Way

Signals - Fixed Time Cycle Time = 65 seconds (Optimum Cycle Time - Minimum Delay)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veb/b	HV %	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Average Speed km/b	
South: P	Pendle V	Way	/0	10	000		VCII				KIT/T	
2	Т	267	2.0	0.226	6.0	LOS A	3.8	26.8	0.48	0.40	42.4	
3	R	582	2.0	0.831	31.2	LOS C	15.9	113.0	0.97	1.10	28.5	
Approac	h	849	2.0	0.831	23.3	LOS B	15.9	113.0	0.81	0.88	31.8	
East: Du	inmore	Street										
4	L	345	2.0	0.331	14.5	LOS A	5.8	41.5	0.57	0.75	37.1	
6	R	297	2.0	0.811	38.7	LOS C	10.4	73.8	1.00	0.96	25.9	
Approac	h	642	2.0	0.811	25.7	LOS B	10.4	73.8	0.77	0.85	30.9	
North: P	endle V	Vay										
7	L	461	2.0	0.548	16.5	LOS B	9.1	64.6	0.66	0.79	35.8	
8	Т	213	2.0	0.449	23.0	LOS B	5.9	42.0	0.89	0.74	30.8	
Approac	h	674	2.0	0.548	18.5	LOS B	9.1	64.6	0.73	0.77	34.1	
All Vehic	les	2165	2.0	0.831	22.5	LOS B	15.9	113.0	0.77	0.84	32.2	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians													
MoviD	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective					
	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate					
		ped/h	sec		ped	m		per ped					
P1	Across S approach	53	11.1	LOS B	0.1	0.1	0.58	0.58					
P3	Across E approach	53	26.8	LOS C	0.1	0.1	0.91	0.91					
All Pede	estrians	106	18.9	LOS B			0.75	0.75					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.



Dunmore Street-Pendle Way

Signals - Fixed Time Cycle Time = 60 seconds (Optimum Cycle Time - Minimum Delay)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h	
South: P	endle \	Way										
2	Т	313	2.0	0.348	11.1	LOS A	5.9	41.9	0.68	0.58	38.0	
3	R	287	2.0	0.672	24.8	LOS B	7.4	52.9	0.96	0.86	31.3	
Approac	h	600	2.0	0.672	17.7	LOS B	7.4	52.9	0.81	0.71	34.4	
East: Du	inmore	Street										
4	L	475	2.0	0.486	16.0	LOS B	8.8	62.9	0.67	0.79	36.1	
6	R	449	2.0	0.736	27.8	LOS B	12.6	90.1	0.94	0.89	30.0	
Approac	h	924	2.0	0.736	21.7	LOS B	12.6	90.1	0.80	0.84	32.8	
North: P	endle V	Vay										
7	L	225	2.0	0.209	9.7	LOS A	2.2	16.0	0.36	0.69	40.5	
8	Т	213	2.0	0.414	20.1	LOS B	5.3	37.7	0.87	0.72	32.3	
Approac	h	438	2.0	0.414	14.8	LOS B	5.3	37.7	0.61	0.71	36.0	
All Vehic	les	1962	2.0	0.736	18.9	LOS B	12.6	90.1	0.76	0.77	34.0	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians													
MaxID	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective					
	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate					
		ped/h	sec		ped	m		per ped					
P1	Across S approach	53	12.0	LOS B	0.1	0.1	0.63	0.63					
P3	Across E approach	53	24.3	LOS C	0.1	0.1	0.90	0.90					
All Pede	estrians	106	18.2	LOS B			0.77	0.77					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.



13S1210100

Dunmore Street-Pendle Way

Signals - Fixed Time Cycle Time = 50 seconds (Optimum Cycle Time - Minimum Delay)

Movem	Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back c Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h		
South: P	Pendle V	Vay											
2	Т	342	2.0	0.520	6.7	LOS A	4.9	35.2	0.58	0.49	41.3		
3	R	245	2.0	0.520	17.1	LOS B	4.9	35.2	0.84	0.81	35.5		
Approac	h	587	2.0	0.520	11.0	LOS A	4.9	35.2	0.69	0.63	38.7		
East: Du	inmore	Street											
4	L	287	2.0	0.357	16.7	LOS B	4.8	33.9	0.71	0.78	35.7		
6	R	268	2.0	0.733	29.7	LOS C	6.9	49.2	0.99	0.92	29.1		
Approac	h	556	2.0	0.733	23.0	LOS B	6.9	49.2	0.84	0.84	32.2		
North: P	endle V	Vay											
7	L	256	2.0	0.238	10.5	LOS A	2.6	18.7	0.44	0.71	39.9		
8	Т	251	2.0	0.407	14.8	LOS B	4.9	35.1	0.83	0.69	35.3		
Approac	h	506	2.0	0.407	12.6	LOS A	4.9	35.1	0.63	0.70	37.5		
All Vehic	les	1649	2.0	0.733	15.5	LOS B	6.9	49.2	0.72	0.72	35.9		

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	Movement Performance - Pedestrians													
	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective						
	Description	FIOW	Delay	Service	Pedestrian	Distance	Queued	Stop Rate						
		pea/n	sec		ped	m		per ped						
P1	Across S approach	53	14.4	LOS B	0.1	0.1	0.76	0.76						
P3	Across E approach	53	19.4	LOS B	0.1	0.1	0.88	0.88						
All Pede	estrians	106	16.9	LOS B			0.82	0.82						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.



Movement Performance - Vehicles													
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h		
South: J	lones St	treet											
2	Т	359	2.0	0.195	1.8	LOS A	1.6	11.4	0.52	0.00	44.1		
3	R	8	2.0	0.195	8.6	LOS A	1.6	11.4	0.52	0.91	43.0		
Approac	ch	367	2.0	0.195	2.0	NA	1.6	11.4	0.52	0.02	44.0		
East: Ro	ogers St	treet											
4	L	13	2.0	0.042	11.8	LOS A	0.1	1.0	0.53	0.66	38.9		
6	R	7	2.0	0.042	12.1	LOS A	0.1	1.0	0.53	0.84	38.8		
Approac	h	20	2.0	0.042	11.9	LOS A	0.1	1.0	0.53	0.73	38.9		
North: J	ones St	reet											
7	L	6	2.0	0.173	6.5	LOS A	0.0	0.0	0.00	0.92	43.3		
8	Т	326	2.0	0.173	0.0	LOS A	0.0	0.0	0.00	0.00	50.0		
Approac	h	333	2.0	0.173	0.1	NA	0.0	0.0	0.00	0.02	49.9		
All Vehic	cles	720	2.0	0.195	1.4	NA	1.6	11.4	0.28	0.04	46.4		

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Project: \\GTA-SYD-SS1\Project_Files\13S1200-1299\13S1210100 Bonds Pendle Hill Amended Report\Modelling \140326sid - 13S1210100 Bonds Spinning Mills - 2027 + Dev.sip

Movem	Movement Performance - Vehicles													
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h			
South: J	lones S	street												
2	Т	345	2.0	0.200	2.0	LOS A	1.6	11.7	0.53	0.00	43.9			
3	R	21	2.0	0.200	8.8	LOS A	1.6	11.7	0.53	0.90	42.8			
Approac	h	366	2.0	0.200	2.4	NA	1.6	11.7	0.53	0.05	43.8			
East: Ro	ogers S	treet												
4	L	25	2.0	0.071	11.4	LOS A	0.2	1.7	0.52	0.69	39.2			
6	R	11	2.0	0.071	11.7	LOS A	0.2	1.7	0.52	0.85	39.1			
Approac	h	36	2.0	0.071	11.5	LOS A	0.2	1.7	0.52	0.73	39.2			
North: J	ones S	treet												
7	L	9	2.0	0.182	6.5	LOS A	0.0	0.0	0.00	0.91	43.3			
8	Т	340	2.0	0.182	0.0	LOS A	0.0	0.0	0.00	0.00	50.0			
Approac	h	349	2.0	0.182	0.2	NA	0.0	0.0	0.00	0.02	49.8			
All Vehic	cles	752	2.0	0.200	1.8	NA	1.6	11.7	0.28	0.07	46.1			

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h	
South: J	ones St	reet										
2	Т	251	2.0	0.151	1.6	LOS A	1.1	7.9	0.47	0.00	44.4	
3	R	22	2.0	0.151	8.4	LOS A	1.1	7.9	0.47	0.88	42.9	
Approac	h	273	2.0	0.151	2.1	NA	1.1	7.9	0.47	0.07	44.3	
East: Ro	ogers St	reet										
4	L	28	2.0	0.069	10.1	LOS A	0.2	1.7	0.47	0.66	40.2	
6	R	13	2.0	0.069	10.5	LOS A	0.2	1.7	0.47	0.82	40.0	
Approac	h	41	2.0	0.069	10.2	LOS A	0.2	1.7	0.47	0.71	40.1	
North: J	ones St	reet										
7	L	21	2.0	0.162	6.5	LOS A	0.0	0.0	0.00	0.89	43.3	
8	Т	291	2.0	0.162	0.0	LOS A	0.0	0.0	0.00	0.00	50.0	
Approac	h	312	2.0	0.162	0.4	NA	0.0	0.0	0.00	0.06	49.5	
All Vehic	cles	625	2.0	0.162	1.8	NA	1.1	7.9	0.24	0.11	46.4	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Movem	Movement Performance - Vehicles													
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h			
South: J	lones Str	reet												
2	Т	385	2.0	0.209	4.0	LOS A	2.3	16.1	0.69	0.00	42.4			
3	R	6	2.0	0.209	10.8	LOS A	2.3	16.1	0.69	1.00	41.7			
Approac	ch	392	2.0	0.209	4.1	NA	2.3	16.1	0.69	0.02	42.4			
East: Oa	atlands S	Street												
4	L	16	2.0	0.129	19.9	LOS B	0.4	3.0	0.75	0.89	33.7			
6	R	16	2.0	0.129	20.3	LOS B	0.4	3.0	0.75	0.92	33.6			
Approac	:h	32	2.0	0.129	20.1	LOS B	0.4	3.0	0.75	0.90	33.7			
North: Jo	ones Str	eet												
7	L	12	2.0	0.294	6.5	LOS A	0.0	0.0	0.00	0.91	43.3			
8	Т	554	2.0	0.294	0.0	LOS A	0.0	0.0	0.00	0.00	50.0			
Approac	h	565	2.0	0.294	0.1	NA	0.0	0.0	0.00	0.02	49.8			
All Vehic	cles	988	2.0	0.294	2.4	NA	2.3	16.1	0.30	0.05	45.9			

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Movem	Movement Performance - Vehicles													
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h			
South: J	lones Str	eet												
2	Т	446	2.0	0.255	4.0	LOS A	2.7	19.5	0.70	0.00	42.2			
3	R	18	2.0	0.255	10.8	LOS A	2.7	19.5	0.70	1.00	41.6			
Approac	ch	464	2.0	0.255	4.3	NA	2.7	19.5	0.70	0.04	42.2			
East: Oa	atlands S	street												
4	L	27	2.0	0.314	26.4	LOS B	1.2	8.3	0.81	0.99	30.5			
6	R	38	2.0	0.314	26.7	LOS B	1.2	8.3	0.81	0.98	30.4			
Approac	ch	65	2.0	0.314	26.6	LOS B	1.2	8.3	0.81	0.99	30.5			
North: J	ones Stre	eet												
7	L	26	2.0	0.282	6.5	LOS A	0.0	0.0	0.00	0.90	43.3			
8	Т	515	2.0	0.282	0.0	LOS A	0.0	0.0	0.00	0.00	50.0			
Approac	ch	541	2.0	0.282	0.3	NA	0.0	0.0	0.00	0.04	49.6			
All Vehic	cles	1071	2.0	0.314	3.6	NA	2.7	19.5	0.35	0.10	44.5			

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Movem	Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h		
South: J	ones St	treet											
2	Т	434	2.0	0.253	3.7	LOS A	2.6	18.5	0.68	0.00	42.4		
3	R	22	2.0	0.253	10.5	LOS A	2.6	18.5	0.68	0.98	41.8		
Approac	h	456	2.0	0.253	4.1	NA	2.6	18.5	0.68	0.05	42.3		
East: Oa	atland S	treet											
4	L	16	2.0	0.204	23.4	LOS B	0.7	4.9	0.79	0.92	31.9		
6	R	27	2.0	0.204	23.7	LOS B	0.7	4.9	0.79	0.94	31.8		
Approac	h	43	2.0	0.204	23.6	LOS B	0.7	4.9	0.79	0.93	31.8		
North: J	ones St	reet											
7	L	31	2.0	0.269	6.5	LOS A	0.0	0.0	0.00	0.90	43.3		
8	Т	486	2.0	0.269	0.0	LOS A	0.0	0.0	0.00	0.00	50.0		
Approac	h	517	2.0	0.269	0.4	NA	0.0	0.0	0.00	0.05	49.5		
All Vehic	cles	1016	2.0	0.269	3.0	NA	2.6	18.5	0.34	0.09	45.0		

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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13S1210100 Jones Street-Smith Street Roundabout

Movem	Movement Performance - Vehicles											
	-	Demand	111/	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average	
MOV ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed	
Ocutha		veh/h	%	V/C	sec		veh	m		per veh	km/h	
South: J	ones Str	eet										
1	L	5	2.0	0.369	7.7	LOS A	2.4	17.3	0.54	0.67	42.0	
2	Т	238	2.0	0.369	6.8	LOS A	2.4	17.3	0.54	0.61	42.1	
3	R	101	2.0	0.369	10.8	LOS A	2.4	17.3	0.54	0.78	40.3	
Approad	h	344	2.0	0.369	8.0	LOS A	2.4	17.3	0.54	0.66	41.5	
East: Sr	nith Stree	et										
4	L	28	2.0	0.285	9.0	LOS A	1.8	12.8	0.68	0.75	41.0	
5	Т	56	2.0	0.285	8.1	LOS A	1.8	12.8	0.68	0.71	41.1	
6	R	123	2.0	0.285	12.1	LOS A	1.8	12.8	0.68	0.82	39.2	
Approac	h	207	2.0	0.285	10.6	LOS A	1.8	12.8	0.68	0.78	39.9	
North: J	ones Stre	eet										
7	L	256	2.0	0.826	18.7	LOS B	13.2	94.3	1.00	1.19	34.7	
8	Т	345	2.0	0.826	17.7	LOS B	13.2	94.3	1.00	1.19	34.8	
9	R	42	2.0	0.826	21.7	LOS B	13.2	94.3	1.00	1.19	33.6	
Approad	h	643	2.0	0.826	18.4	LOS B	13.2	94.3	1.00	1.19	34.7	
West: S	mith Stre	et										
10	L	59	2.0	0.494	11.1	LOS A	3.6	25.9	0.75	0.88	39.8	
11	Т	285	2.0	0.494	10.1	LOS A	3.6	25.9	0.75	0.85	40.1	
12	R	17	2.0	0.494	14.1	LOS A	3.6	25.9	0.75	0.95	38.2	
Approac	h	361	2.0	0.494	10.4	LOS A	3.6	25.9	0.75	0.86	40.0	
All Vehic	cles	1556	2.0	0.826	13.2	LOS A	13.2	94.3	0.80	0.94	37.9	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.



13S1210100 Jones Street-Smith Street Roundabout

Movem	Movement Performance - Vehicles Demand Deg. Average Level of 95% Back of Queue Prop. Effective Average													
Mov ID	Turn	Demand Flow	HV %	Deg. Satn	Average Delay	Level of Service	95% Back (Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed			
South: J	ones Str	eet	70	V/C	Sec	_	ven		_	per ven	K111/11			
1	L	33	2.0	0.450	10.0	LOS A	3.1	22.0	0.73	0.83	40.7			
2	т	267	2.0	0.450	9.0	LOS A	3.1	22.0	0.73	0.79	40.9			
3	R	33	2.0	0.450	13.0	LOS A	3.1	22.0	0.73	0.90	38.9			
Approac	h	333	2.0	0.450	9.5	LOS A	3.1	22.0	0.73	0.81	40.7			
East: Sr	nith Stree	et												
4	L	65	2.0	0.501	8.8	LOS A	3.6	25.8	0.67	0.75	41.4			
5	Т	236	2.0	0.501	7.9	LOS A	3.6	25.8	0.67	0.70	41.4			
6	R	131	2.0	0.501	11.9	LOS A	3.6	25.8	0.67	0.82	39.5			
Approad	h	432	2.0	0.501	9.2	LOS A	3.6	25.8	0.67	0.75	40.8			
North: J	ones Stre	eet												
7	L	199	2.0	0.460	7.3	LOS A	3.4	24.6	0.50	0.62	42.1			
8	Т	226	2.0	0.460	6.3	LOS A	3.4	24.6	0.50	0.56	42.2			
9	R	61	2.0	0.460	10.3	LOS A	3.4	24.6	0.50	0.74	40.6			
Approad	h	486	2.0	0.460	7.2	LOS A	3.4	24.6	0.50	0.61	41.9			
West: S	mith Stre	et												
10	L	60	2.0	0.248	9.2	LOS A	1.4	10.2	0.64	0.76	41.2			
11	Т	112	2.0	0.248	8.2	LOS A	1.4	10.2	0.64	0.71	41.5			
12	R	13	2.0	0.248	12.2	LOS A	1.4	10.2	0.64	0.85	39.3			
Approac	h	184	2.0	0.248	8.8	LOS A	1.4	10.2	0.64	0.73	41.3			
All Vehic	cles	1435	2.0	0.501	8.6	LOS A	3.6	25.8	0.62	0.71	41.2			

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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13S1210100 Jones Street-Smith Street Roundabout

Movem	Movement Performance - Vehicles Demand Deg. Average Level of 95% Back of Queue Prop. Effective Average													
		Demand	1.0.7	Deg.	Average	Level of	95% Back o	of Queue	Prop.	Effective	Average			
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed			
0 11 1	01	veh/h	%	v/c	sec		veh	m		per veh	km/h			
South: J	ones Str	eet												
1	L	22	2.0	0.366	8.3	LOS A	2.3	16.4	0.59	0.71	41.9			
2	Т	255	2.0	0.366	7.4	LOS A	2.3	16.4	0.59	0.66	41.9			
3	R	38	2.0	0.366	11.4	LOS A	2.3	16.4	0.59	0.82	40.0			
Approac	h	315	2.0	0.366	7.9	LOS A	2.3	16.4	0.59	0.68	41.7			
East: Sn	nith Stree	et												
4	L	20	2.0	0.295	8.5	LOS A	1.8	12.5	0.59	0.71	41.5			
5	Т	94	2.0	0.295	7.5	LOS A	1.8	12.5	0.59	0.66	41.6			
6	R	128	2.0	0.295	11.6	LOS A	1.8	12.5	0.59	0.79	39.6			
Approac	h	242	2.0	0.295	9.8	LOS A	1.8	12.5	0.59	0.73	40.5			
North: Jo	ones Stre	eet												
7	L	183	2.0	0.475	7.4	LOS A	3.6	25.5	0.52	0.63	42.0			
8	Т	249	2.0	0.475	6.4	LOS A	3.6	25.5	0.52	0.57	42.1			
9	R	63	2.0	0.475	10.5	LOS A	3.6	25.5	0.52	0.74	40.5			
Approac	h	496	2.0	0.475	7.3	LOS A	3.6	25.5	0.52	0.61	41.9			
West: Si	mith Stre	et												
10	L	54	2.0	0.240	9.1	LOS A	1.4	9.6	0.62	0.75	41.2			
11	Т	112	2.0	0.240	8.1	LOS A	1.4	9.6	0.62	0.70	41.6			
12	R	17	2.0	0.240	12.2	LOS A	1.4	9.6	0.62	0.84	39.4			
Approac	h	182	2.0	0.240	8.8	LOS A	1.4	9.6	0.62	0.73	41.3			
All Vehic	les	1235	2.0	0.475	8.2	LOS A	3.6	25.5	0.56	0.67	41.5			

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.



13S1210100 Jones Street-Dunmore Street Roundabout

Moven	nent Per	formance	- Vehicles								
	T	Demand		Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
South	longo Str	veh/h	%	V/C	sec		veh	m		per veh	km/h
	Jones Su	400	0.0	0.540	40.0		4.0	00.0	0.70	0.07	00.4
1	L	199	2.0	0.510	10.9	LOSA	4.0	28.3	0.78	0.87	39.4
2	-	1/	2.0	0.510	10.0	LOSA	4.0	28.3	0.78	0.85	39.7
3	R	152	2.0	0.510	14.0	LOS A	4.0	28.3	0.78	0.92	37.8
Approa	ch	367	2.0	0.510	12.2	LOS A	4.0	28.3	0.78	0.89	38.8
East: D	unmore S	Street									
4	L	78	2.0	0.575	10.0	LOS A	5.1	36.1	0.76	0.82	40.7
5	Т	388	2.0	0.575	9.0	LOS A	5.1	36.1	0.76	0.79	41.0
6	R	13	2.0	0.575	13.0	LOS A	5.1	36.1	0.76	0.88	39.0
Approa	ch	479	2.0	0.575	9.3	LOS A	5.1	36.1	0.76	0.80	40.9
North: J	ones Str	eet									
7	L	5	2.0	0.358	22.7	LOS B	2.5	17.6	1.00	1.02	32.3
8	Т	39	2.0	0.358	21.7	LOS B	2.5	17.6	1.00	1.02	32.4
9	R	39	2.0	0.358	25.7	LOS B	2.5	17.6	1.00	1.02	31.5
Approa	ch	83	2.0	0.358	23.7	LOS B	2.5	17.6	1.00	1.02	32.0
West: D	unmore	Street									
10	L	24	2.0	0.977	27.4	LOS B	40.1	285.9	1.00	1.11	30.3
11	Т	785	2.0	0.977	26.4	LOS B	40.1	285.9	1.00	1.11	30.4
12	R	239	2.0	0.977	30.4	LOS C	40.1	285.9	1.00	1.11	29.6
Approa	ch	1048	2.0	0.977	27.3	LOS B	40.1	285.9	1.00	1.11	30.2
All Vehi	cles	1978	2.0	0.977	20.0	LOS B	40.1	285.9	0.90	0.99	33.8

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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13S1210100 Jones Street-Dunmore Street Roundabout

Moven	nent Per	formance	- Vehicles								
Mov ID	Turn	Demand Flow	HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
South	longo Str	veh/h	%	V/C	sec		veh	m		per veh	km/h
300011. 0		220	2.0	0 725	22.7		0.1	57.0	1 00	1.00	22.2
1	L -	229	2.0	0.735	22.7	LUSB	8.1	57.9	1.00	1.23	32.3
2	1	59	2.0	0.735	21.7	LUSB	8.1	57.9	1.00	1.23	32.4
3	. к	67	2.0	0.735	25.7	LUSB	8.1	57.9	1.00	1.23	31.5
Approa	ch	356	2.0	0.735	23.1	LOS B	8.1	57.9	1.00	1.23	32.2
East: D	unmore S	Street									
4	L	87	2.0	0.839	16.2	LOS B	14.8	105.6	1.00	1.07	36.4
5	Т	646	2.0	0.839	15.3	LOS B	14.8	105.6	1.00	1.06	36.4
6	R	25	2.0	0.839	19.3	LOS B	14.8	105.6	1.00	1.07	35.2
Approa	ch	759	2.0	0.839	15.5	LOS B	14.8	105.6	1.00	1.06	36.4
North: J	ones Stre	eet									
7	L	13	2.0	0.138	10.4	LOS A	0.8	5.5	0.69	0.77	39.9
8	Т	20	2.0	0.138	9.4	LOS A	0.8	5.5	0.69	0.74	40.2
9	R	53	2.0	0.138	13.5	LOS A	0.8	5.5	0.69	0.83	38.2
Approa	ch	85	2.0	0.138	12.1	LOS A	0.8	5.5	0.69	0.80	38.9
West: D	unmore	Street									
10	L	62	2.0	0.565	7.4	LOS A	5.1	36.6	0.59	0.62	41.7
11	Т	314	2.0	0.565	6.5	LOS A	5.1	36.6	0.59	0.57	41.7
12	R	228	2.0	0.565	10.5	LOS A	5.1	36.6	0.59	0.72	40.4
Approa	ch	604	2.0	0.565	8.1	LOS A	5.1	36.6	0.59	0.63	41.2
All Vehi	cles	1804	2.0	0.839	14.4	LOS A	14.8	105.6	0.85	0.94	37.0

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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13S1210100 Jones Street-Dunmore Street Roundabout

Movement Performanc		formance ·	- Vehicles								
	T	Demand	1.15.7	Deg.	Average	Level of	95% Back o	of Queue	Prop.	Effective	Average
MOV ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
Ocutha	lanaa Ota	veh/h	%	V/C	sec		veh	m		per veh	km/h
South:	Jones Str	eet									
1	L	169	2.0	0.366	10.1	LOS A	2.3	16.5	0.74	0.82	40.1
2	Т	21	2.0	0.366	9.2	LOS A	2.3	16.5	0.74	0.79	40.4
3	R	58	2.0	0.366	13.2	LOS A	2.3	16.5	0.74	0.88	38.4
Approa	ch	248	2.0	0.366	10.8	LOS A	2.3	16.5	0.74	0.83	39.7
East: D	unmore S	Street									
4	L	63	2.0	0.580	9.3	LOS A	4.9	34.8	0.69	0.77	41.3
5	Т	448	2.0	0.580	8.3	LOS A	4.9	34.8	0.69	0.73	41.5
6	R	17	2.0	0.580	12.4	LOS A	4.9	34.8	0.69	0.85	39.4
Approa	ch	528	2.0	0.580	8.6	LOS A	4.9	34.8	0.69	0.74	41.4
North: J	lones Stre	eet									
7	L	18	2.0	0.132	10.8	LOS A	0.7	5.2	0.70	0.78	39.6
8	Т	24	2.0	0.132	9.8	LOS A	0.7	5.2	0.70	0.74	39.9
9	R	38	2.0	0.132	13.9	LOS A	0.7	5.2	0.70	0.84	38.0
Approa	ch	80	2.0	0.132	12.0	LOS A	0.7	5.2	0.70	0.80	38.9
West: D	ounmore \$	Street									
10	L	38	2.0	0.530	6.8	LOS A	4.7	33.5	0.44	0.57	42.2
11	Т	380	2.0	0.530	5.8	LOS A	4.7	33.5	0.44	0.50	42.5
12	R	214	2.0	0.530	9.8	LOS A	4.7	33.5	0.44	0.70	40.7
Approa	ch	632	2.0	0.530	7.2	LOSA	4.7	33.5	0.44	0.57	41.8
All Vehi	cles	1488	2.0	0.580	8.6	LOS A	4.9	34.8	0.60	0.69	41.2

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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Dunmore Street-Goodall Street

Signals - Fixed Time Cycle Time = 90 seconds (Optimum Cycle Time - Minimum Delay)

Movem	ent Pe	erformance - '	Vehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
East: Du	inmore	Street									
5	Т	356	2.0	0.439	8.4	LOS A	7.1	50.9	0.50	0.43	40.4
6	R	316	2.0	0.847	52.4	LOS D	13.7	97.3	1.00	1.13	22.1
Approac	h	672	2.0	0.847	29.1	LOS C	13.7	97.3	0.73	0.76	29.1
North: G	oodall :	Street									
7	L	333	2.0	0.859	34.6	LOS C	11.5	81.6	0.82	0.89	27.3
9	R	461	2.0	1.030	107.8	LOS F	35.6	253.5	1.00	1.32	13.9
Approac	h	794	2.0	1.030	77.1	LOS F	35.6	253.5	0.93	1.14	17.5
West: D	unmore	Street									
10	L	376	2.0	0.410	11.5	LOS A	5.9	42.1	0.39	0.71	39.2
11	Т	773	2.0	1.003	79.8	LOS F	56.0	398.9	1.00	1.44	16.6
Approac	h	1148	2.0	1.003	57.4	LOS E	56.0	398.9	0.80	1.20	20.5
All Vehic	les	2614	2.0	1.030	56.1	LOS D	56.0	398.9	0.82	1.07	21.0

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians												
MoviD	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective				
	Description	FIOW	Delay	Service	Pedestrian	Distance	Queued	Stop Rate				
		ped/h	sec		ped	m		per ped				
P5	Across N approach	53	22.8	LOS C	0.1	0.1	0.71	0.71				
P7	Across W approach	53	33.8	LOS D	0.1	0.1	0.87	0.87				
All Ped	estrians	106	28.3	LOS C			0.79	0.79				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.



Dunmore Street-Goodall Street

Signals - Fixed Time Cycle Time = 56 seconds (Optimum Cycle Time - Minimum Delay)

Movement Performance - Vehicles											
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
East: Du	inmore	Street									
5	Т	595	2.0	0.669	11.1	LOS A	11.9	84.9	0.78	0.69	37.8
6	R	327	2.0	0.874	36.7	LOS C	9.7	68.9	1.00	1.14	26.5
Approac	h	922	2.0	0.874	20.2	LOS B	11.9	84.9	0.86	0.85	32.8
North: G	oodall	Street									
7	L	325	2.0	0.570	15.7	LOS B	5.5	38.9	0.65	0.76	36.3
9	R	449	2.0	0.859	35.2	LOS C	14.5	103.4	1.00	1.03	27.0
Approac	h	775	2.0	0.859	27.0	LOS B	14.5	103.4	0.85	0.92	30.3
West: Du	unmore	Street									
10	L	262	2.0	0.243	10.1	LOS A	2.7	19.1	0.40	0.71	40.3
11	Т	386	2.0	0.702	21.0	LOS B	10.1	72.3	0.95	0.86	31.7
Approac	h	648	2.0	0.702	16.6	LOS B	10.1	72.3	0.73	0.80	34.7
All Vehic	les	2345	2.0	0.874	21.4	LOS B	14.5	103.4	0.82	0.86	32.4

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians												
MoviD	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective				
	Description	FIOW	Delay	Service	Pedestrian	Distance	Queued	Stop Rate				
		ped/h	sec		ped	m		per ped				
P5	Across N approach	53	22.3	LOS C	0.1	0.1	0.89	0.89				
P7	Across W approach	53	22.3	LOS C	0.1	0.1	0.89	0.89				
All Ped	estrians	106	22.3	LOS C			0.89	0.89				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.



Dunmore Street-Goodall Street

Signals - Fixed Time Cycle Time = 65 seconds (Optimum Cycle Time - Minimum Delay)

Movement Performance - Vehicles											
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
East: Du	Inmore	Street									
5	Т	476	2.0	0.531	8.6	LOS A	8.6	61.6	0.61	0.54	39.9
6	R	291	2.0	0.685	28.1	LOS B	8.2	58.7	0.97	0.92	29.8
Approac	h	766	2.0	0.685	16.0	LOS B	8.6	61.6	0.75	0.68	35.4
North: G	oodall \$	Street									
7	L	283	2.0	0.580	18.5	LOS B	5.8	41.1	0.68	0.77	34.6
9	R	237	2.0	0.525	30.1	LOS C	6.7	47.9	0.92	0.81	29.0
Approac	:h	520	2.0	0.580	23.8	LOS B	6.7	47.9	0.79	0.79	31.8
West: D	unmore	Street									
10	L	186	2.0	0.217	12.0	LOS A	2.3	16.3	0.60	0.74	38.9
11	Т	462	2.0	0.665	19.3	LOS B	12.4	88.4	0.90	0.78	32.6
Approac	h	648	2.0	0.665	17.2	LOS B	12.4	88.4	0.81	0.77	34.2
All Vehic	les	1935	2.0	0.685	18.5	LOS B	12.4	88.4	0.78	0.74	33.9

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians												
Movin	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective				
	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate				
		ped/h	sec		ped	m		per ped				
P5	Across N approach	53	20.8	LOS C	0.1	0.1	0.80	0.80				
P7	Across W approach	53	26.8	LOS C	0.1	0.1	0.91	0.91				
All Ped	estrians	106	23.8	LOS C			0.85	0.85				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.



Dunmore Street-Pendle Way

Signals - Fixed Time Cycle Time = 75 seconds (Optimum Cycle Time - Minimum Delay)

Movement Performance - Vehicles											
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: P	endle V	Vay									
2	Т	267	2.0	0.217	6.0	LOS A	4.0	28.8	0.45	0.38	42.5
3	R	646	2.0	0.817	31.1	LOS C	18.0	128.0	0.94	1.07	28.6
Approac	h	914	2.0	0.817	23.8	LOS B	18.0	128.0	0.80	0.87	31.6
East: Du	nmore	Street									
4	L	394	2.0	0.343	13.6	LOS A	6.8	48.5	0.51	0.74	37.7
6	R	301	2.0	0.822	43.6	LOS D	12.1	86.0	1.00	0.96	24.4
Approac	h	695	2.0	0.822	26.6	LOS B	12.1	86.0	0.72	0.84	30.5
North: P	endle V	Vay									
7	L	471	2.0	0.639	20.5	LOS B	11.8	83.9	0.73	0.80	33.5
8	Т	213	2.0	0.518	29.0	LOS C	7.1	50.5	0.93	0.77	28.3
Approac	h	683	2.0	0.639	23.1	LOS B	11.8	83.9	0.79	0.79	31.7
All Vehic	les	2292	2.0	0.822	24.4	LOS B	18.0	128.0	0.77	0.84	31.3

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians													
	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective					
	Description	ped/h	sec	Service	pedesthan	Distance	Queuea	per ped					
P1	Across S approach	53	9.6	LOS A	0.1	0.1	0.51	0.51					
P3	Across E approach	53	31.7	LOS D	0.1	0.1	0.92	0.92					
All Pede	estrians	106	20.7	LOS C			0.71	0.71					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.



Dunmore Street-Pendle Way Signals - Fixed Time Cycle Time = 65 seconds (Optimum Cycle Time - Minimum Delay)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back (Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h	
South: P	endle V	Vay										
2	Т	313	2.0	0.330	10.8	LOS A	6.0	43.0	0.65	0.55	38.2	
3	R	395	2.0	0.794	31.9	LOS C	11.6	82.8	0.99	1.04	28.3	
Approac	h	707	2.0	0.794	22.6	LOS B	11.6	82.8	0.84	0.82	31.9	
East: Du	inmore	Street										
4	L	552	2.0	0.529	15.8	LOS B	10.8	77.2	0.66	0.79	36.2	
6	R	461	2.0	0.779	31.5	LOS C	14.8	105.1	0.97	0.92	28.4	
Approac	h	1013	2.0	0.779	23.0	LOS B	14.8	105.1	0.80	0.85	32.2	
North: P	endle V	Vay										
7	L	242	2.0	0.245	11.0	LOS A	3.0	21.2	0.41	0.71	39.5	
8	Т	213	2.0	0.449	23.0	LOS B	5.9	42.0	0.89	0.74	30.8	
Approac	h	455	2.0	0.449	16.7	LOS B	5.9	42.0	0.64	0.72	34.9	
All Vehic	les	2175	2.0	0.794	21.5	LOS B	14.8	105.1	0.78	0.82	32.6	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians													
	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective					
	Description	FIOW	Delay	Service	Pedestrian	Distance	Queued	Stop Rate					
		ped/h	sec		ped	m		per ped					
P1	Across S approach	53	11.1	LOS B	0.1	0.1	0.58	0.58					
P3	Across E approach	53	26.8	LOS C	0.1	0.1	0.91	0.91					
All Pede	estrians	106	18.9	LOS B			0.75	0.75					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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13S1210100

Dunmore Street-Pendle Way

Signals - Fixed Time Cycle Time = 50 seconds (Optimum Cycle Time - Minimum Delay)

Movement Performance - Vehicles													
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Average Speed km/h		
South: P	Pendle \	Way	,0		000		Volt				TXT 0 TT		
2	Т	342	2.0	0.317	6.4	LOS A	4.5	32.1	0.57	0.49	41.8		
3	R	382	2.0	0.755	22.1	LOS B	8.6	61.4	0.97	0.94	32.6		
Approac	h	724	2.0	0.755	14.7	LOS B	8.6	61.4	0.78	0.73	36.4		
East: Du	inmore	Street											
4	L	424	2.0	0.527	17.7	LOS B	7.7	55.1	0.78	0.81	35.1		
6	R	289	2.0	0.790	31.5	LOS C	7.8	55.8	1.00	0.97	28.4		
Approac	h	714	2.0	0.790	23.3	LOS B	7.8	55.8	0.87	0.87	32.0		
North: P	endle V	Vay											
7	L	277	2.0	0.258	10.6	LOS A	2.9	20.5	0.45	0.72	39.9		
8	Т	251	2.0	0.407	14.8	LOS B	4.9	35.1	0.83	0.69	35.3		
Approac	h	527	2.0	0.407	12.6	LOS A	4.9	35.1	0.63	0.70	37.6		
All Vehic	les	1965	2.0	0.790	17.3	LOS B	8.6	61.4	0.77	0.77	35.0		

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians												
	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective				
	Description	FIOW	Delay	Service	Pedestrian	Distance	Queued	Stop Rate				
		ped/h	sec		ped	m		per ped				
P1	Across S approach	53	14.4	LOS B	0.1	0.1	0.76	0.76				
P3	Across E approach	53	19.4	LOS B	0.1	0.1	0.88	0.88				
All Pede	estrians	106	16.9	LOS B			0.82	0.82				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Attachment 2

Attachment 2A - Existing Peak Hour Intersection Turning Movement Diagrams for Thursday PM & Saturday Midday

Attachment 2B – 2027 Future Base Peak Hour Intersection Turning Movement Diagrams for Thursday PM & Saturday Midday

Attachment 2C – 2027 Post Development Peak Hour Intersection Turning Movement Diagrams for Thursday PM & Saturday Midday



Attachment 2A - Existing Peak Hour Intersection Turning Movement Diagrams for Thursday PM & Saturday Midday







Attachment 2B – 2027 Future Base Peak Hour Intersection Turning Movement Diagrams for Thursday PM & Saturday Midday







Attachment 2C – 2027 Post Development Peak Hour Intersection Turning Movement Diagrams for Thursday PM & Saturday Midday











Attachment 3

Attachment 3A - SIDRA Output (Existing Thursday PM & Saturday Midday)

Attachment 3B - SIDRA Output (2027 Future Base Thursday PM & Saturday Midday)

Attachment 3C – SIDRA Output (2027 Post Development Thursday PM & Saturday Midday)



Attachment 3A – SIDRA Output (Existing Thursday PM & Saturday Midday)

Intersection	Peak	Leg	Degree of Saturation (DOS)	Average Delay (sec)	95 th Percentile	Level of Service (LOS)
		South	0.14	1	7	А
		East	0.02	8	0	А
Intersection Jones Street/ Rogers Street/ Oatlands Street Jones Street/ Smith Street Jones Street/ Dunmore Street/	PIVI	North	0.14	0	0	А
Jones Street/		All	0.14	1	7	А
Rogers Street		South	0.09	1	4	А
	Cat	East	0.02	7	0	А
	ડ્યા	North	0.11	0	0	А
		All	0.11	1	4	А
		South	0.13	2	6	А
	DM	East	0.03	8	1	А
	PIVI	North	0.16	0	0	А
Jones Street/		All	0.16	1	6	А
Oatlands Street		South	0.09	1	4	A
	C - t	East	0.02	7	0	А
	Sat	North	0.11	0	0	A
		All	0.11	1	4	А
		South	0.25	11	10	А
		East	0.32	10	14	А
	PM	North	0.25	10	10	А
		West	0.15	11	6	А
Jones Street/		All	0.32	7	14	А
Smith Street		South	0.16	10	6	А
		East	0.14	10	5	А
	Sat	North	0.21	10	8	А
		West	0.13	10	5	А
		All	0.21	7	8	А
		South	0.43	15	21	А
		East	0.63	12	43	А
	PM	North	0.10	12	4	А
		West	0.41	10	22	А
Jones Street/		All	0.63	9	43	A
Dunmore Street		South	0.20	12	8	А
		East	0.38	11	17	A
	Sat	North	0.09	12	3	A
		West	0.36	9	18	A
		All	0.38	7	18	A



Intersection	Peak	Leg	Degree of Saturation (DOS)	Average Delay (sec)	95 th Percentile	Level of Service (LOS)
		East	0.57	14	57	А
	DM	North	0.76	24	79	В
	PIVI	West	0.42	14	38	А
Goodall Street/		All	0.76	18	79	В
Dunmore Street		East	0.45	14	32	А
	Cat	North	0.40	20	34	В
	581	West	0.48	15	44	В
		All	0.48	16	44	В
		South	0.57	16	43	В
	DM	East	0.65	20	73	В
	PM	North	0.37	15	33	В
Pendle Way/		All	0.65	18	73	В
Dunmore Street		South	0.45	10	30	А
	<u> </u>	East	0.65	22	41	В
	Sat	North	0.36	12	31	А
		All	0.65	15	41	В
		South	0.43	62	31	E
		East	0.51	14	157	А
	PM	North	0.62	60	50	E
		West	0.42	22	122	В
Great Western		All	0.62	20	157	В
Highway/ Jones Street		South	0.59	60	33	E
		East	0.34	19	102	В
	Sat	North	0.59	56	31	D
		West	0.39	19	102	В
		All	0.59	23	102	В
		South	0.70	37	244	С
		East	0.83	40	187	С
	PM	North	0.75	35	276	С
		West	0.53	57	78	E
Cumberland		All	0.83	38	276	С
Hignway/ Dunmore Street		South	0.71	38	177	С
		East	0.86	41	155	С
	Sat	North	0.73	35	183	С
		West	0.52	52	76	D
		All	0.86	38	183	С



Intersection	Peak	Leg	Degree of Saturation (DOS)	Average Delay (sec)	95 th Percentile	Level of Service (LOS)
		South	0.59	11	123	А
		East	0.86	69	121	E
	PM	North	0.59	10	114	А
		West	0.67	67	53	E
Cumberland		All	0.86	16	123	В
Street		South	0.46	14	127	А
		East	0.54	59	46	E
	Sat	North	0.43	13	115	А
		West	0.64	65	45	E
		All	0.64	17	127	В
		South	0.59	16	17	В
	-	North	0.85	41	43	С
	PM	West	0.21	8	0	А
Pendle Way/		All	0.85	15	43	В
Gilba Road		South	0.59	17	18	В
		North	0.86	39	46	С
	Sat	West	0.23	8	0	А
		All	0.86	16	46	В
		South	0.71	29	75	С
	514	East	0.73	30	92	С
	PM	West	0.55	20	112	В
Goodall Street/		All	0.73	26	112	В
Avenue		South	0.86	34	86	С
		East	0.76	32	91	С
	Sat	West	0.47	19	104	В
		All	0.86	27	104	В
		South	0.93	74	101	F
		East	0.90	35	191	С
	PM	North	0.79	59	179	E
		West	0.54	36	135	С
Cumberland		All	0.93	43	191	D
Highway/ Pendle Way		South	0.81	62	93	E
		East	0.99	37	104	С
	Sat	North	0.70	51	106	D
		West	0.53	37	134	С
		All	0.99	42	134	С

13S1210200 Jones Street-Rogers Street Giveway / Yield (Two-Way)

Movem	Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h		
South: J	lones S	Street											
2	Т	255	2.0	0.141	1.0	LOS A	0.9	6.5	0.40	0.00	45.2		
3	R	12	2.0	0.141	7.8	LOS A	0.9	6.5	0.40	0.87	43.0		
Approac	h	266	2.0	0.141	1.3	NA	0.9	6.5	0.40	0.04	45.1		
East: Ro	ogers S	Street											
4	L	12	2.0	0.017	7.9	LOS A	0.1	0.4	0.37	0.58	41.9		
6	R	6	2.0	0.017	8.2	LOS A	0.1	0.4	0.37	0.70	41.8		
Approac	h	18	2.0	0.017	8.0	LOS A	0.1	0.4	0.37	0.62	41.9		
North: J	ones S	treet											
7	L	6	2.0	0.142	6.5	LOS A	0.0	0.0	0.00	0.91	43.3		
8	Т	266	2.0	0.142	0.0	LOS A	0.0	0.0	0.00	0.00	50.0		
Approac	ch	273	2.0	0.142	0.1	NA	0.0	0.0	0.00	0.02	49.8		
All Vehic	cles	557	2.0	0.142	1.0	NA	0.9	6.5	0.20	0.05	47.2		

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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13S1210200 Bonds Spinning Mills - Existing Conditions.sip 8000056, GTA CONSULTANTS, ENTERPRISE 13S1210200 Jones Street-Rogers Street Giveway / Yield (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h	
South: J	lones S	Street										
2	Т	162	2.0	0.088	0.7	LOS A	0.5	3.7	0.33	0.00	46.0	
3	R	5	2.0	0.088	7.5	LOS A	0.5	3.7	0.33	0.87	43.0	
Approac	h	167	2.0	0.088	0.9	NA	0.5	3.7	0.33	0.03	45.9	
East: Ro	ogers S	treet										
4	L	12	2.0	0.015	7.4	LOS A	0.1	0.4	0.31	0.56	42.1	
6	R	6	2.0	0.015	7.7	LOS A	0.1	0.4	0.31	0.66	42.1	
Approac	h	18	2.0	0.015	7.5	LOS A	0.1	0.4	0.31	0.60	42.1	
North: J	ones S	treet										
7	L	14	2.0	0.110	6.5	LOS A	0.0	0.0	0.00	0.89	43.3	
8	Т	198	2.0	0.110	0.0	LOS A	0.0	0.0	0.00	0.00	50.0	
Approac	ch	212	2.0	0.110	0.4	NA	0.0	0.0	0.00	0.06	49.5	
All Vehic	cles	397	2.0	0.110	0.9	NA	0.5	3.7	0.15	0.07	47.5	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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13S1210200 Bonds Spinning Mills - Existing Conditions.sip 8000056, GTA CONSULTANTS, ENTERPRISE 13S1210200 Jones Street-Oatlands Street Giveway / Yield (Two-Way)

Movem	Movement Performance - Vehicles													
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h			
South: J	ones S	street												
2	Т	232	2.0	0.132	1.1	LOS A	0.9	6.1	0.42	0.00	45.0			
3	R	16	2.0	0.132	7.9	LOS A	0.9	6.1	0.42	0.87	43.0			
Approac	h	247	2.0	0.132	1.6	NA	0.9	6.1	0.42	0.06	44.9			
East: Oa	atlands	Street												
4	L	24	2.0	0.032	7.8	LOS A	0.1	0.8	0.38	0.60	41.9			
6	R	9	2.0	0.032	8.2	LOS A	0.1	0.8	0.38	0.72	41.9			
Approac	h	34	2.0	0.032	7.9	LOS A	0.1	0.8	0.38	0.63	41.9			
North: Jo	ones St	treet												
7	L	13	2.0	0.156	6.5	LOS A	0.0	0.0	0.00	0.90	43.3			
8	Т	286	2.0	0.156	0.0	LOS A	0.0	0.0	0.00	0.00	50.0			
Approac	h	299	2.0	0.156	0.3	NA	0.0	0.0	0.00	0.04	49.7			
All Vehic	cles	580	2.0	0.156	1.3	NA	0.9	6.1	0.20	0.08	47.0			

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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13S1210200 Jones Street-Oatlands Street Giveway / Yield (Two-Way)

Movem	Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay s <u>ec</u>	Level of Service	95% Back Vehicles veh	of Queue Distance <u>m</u>	Prop. Queued	Effective Stop Rate per v <u>eh</u>	Average Speed km <u>/h</u>		
South: J	ones S	Street											
2	Т	155	2.0	0.094	0.7	LOS A	0.5	3.9	0.33	0.00	45.9		
3	R	20	2.0	0.094	7.5	LOS A	0.5	3.9	0.33	0.84	42.9		
Approac	h	175	2.0	0.094	1.5	NA	0.5	3.9	0.33	0.10	45.5		
East: Oa	atland S	Street											
4	L	14	2.0	0.016	7.3	LOS A	0.1	0.4	0.30	0.56	42.2		
6	R	5	2.0	0.016	7.7	LOS A	0.1	0.4	0.30	0.67	42.1		
Approac	h	19	2.0	0.016	7.4	LOS A	0.1	0.4	0.30	0.59	42.2		
North: Jo	ones S	treet											
7	L	8	2.0	0.109	6.5	LOS A	0.0	0.0	0.00	0.90	43.3		
8	Т	201	2.0	0.109	0.0	LOS A	0.0	0.0	0.00	0.00	50.0		
Approac	:h	209	2.0	0.109	0.3	NA	0.0	0.0	0.00	0.04	49.7		
All Vehic	cles	403	2.0	0.109	1.1	NA	0.5	3.9	0.16	0.09	47.4		

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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13S1210200 Bonds Spinning Mills - Existing Conditions.sip 8000056, GTA CONSULTANTS, ENTERPRISE

13S1210200 Jones Street-Smith Street Roundabout

Movement Performance - Vehicles													
		Demand		Deg.	Average	Level of	95% Back of	of Queue	Prop.	Effective	Average		
Mov ID	Iurn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed		
Ocuther	Ot	veh/h	%	V/C	sec	_	veh	m		per veh	km/h		
South: J	ones Str	eel	0.0	0.055		1004		40.0	0.55	0.70	40.0		
1	L	29	2.0	0.255	8.2	LOSA	1.4	10.3	0.55	0.70	42.0		
2	I	157	2.0	0.255	7.3	LOS A	1.4	10.3	0.55	0.64	42.1		
3	R	29	2.0	0.255	11.3	LOS A	1.4	10.3	0.55	0.81	40.0		
Approac	h	216	2.0	0.255	8.0	LOS A	1.4	10.3	0.55	0.67	41.8		
East: Sn	nith Stree	et											
4	L	58	2.0	0.318	7.0	LOS A	2.0	14.1	0.42	0.61	42.4		
5	Т	208	2.0	0.318	6.1	LOS A	2.0	14.1	0.42	0.53	42.6		
6	R	64	2.0	0.318	10.1	LOS A	2.0	14.1	0.42	0.75	40.7		
Approac	h	331	2.0	0.318	7.0	LOS A	2.0	14.1	0.42	0.59	42.2		
North: Jo	ones Stre	eet											
7	L	122	2.0	0.252	6.9	LOS A	1.5	10.5	0.38	0.59	42.4		
8	Т	104	2.0	0.252	5.9	LOS A	1.5	10.5	0.38	0.51	42.8		
9	R	35	2.0	0.252	10.0	LOS A	1.5	10.5	0.38	0.73	40.8		
Approac	h	261	2.0	0.252	6.9	LOS A	1.5	10.5	0.38	0.58	42.3		
West: Si	mith Stre	et											
10	L	23	2.0	0.150	7.6	LOS A	0.8	5.6	0.47	0.65	42.3		
11	т	99	2.0	0.150	6.7	LOS A	0.8	5.6	0.47	0.58	42.5		
12	R	12	2.0	0.150	10.7	LOS A	0.8	5.6	0.47	0.78	40.4		
Approac	h	134	2.0	0.150	7.2	LOS A	0.8	5.6	0.47	0.61	42.3		
All Vehic	les	941	2.0	0.318	7.2	LOS A	2.0	14.1	0.44	0.61	42.1		

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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13S1210200 Jones Street-Smith Street Roundabout

Movement Performance - Vehicles													
		Demand		Deg.	Average	Level of	95% Back o	of Queue	Prop.	Effective	Average		
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed		
0	01	veh/h	%	v/c	sec		veh	m		per veh	km/h		
South: J	ones Str	eet											
1	L	20	2.0	0.161	6.9	LOS A	0.9	6.1	0.35	0.60	42.6		
2	Т	107	2.0	0.161	5.9	LOS A	0.9	6.1	0.35	0.51	43.0		
3	R	34	2.0	0.161	9.9	LOS A	0.9	6.1	0.35	0.75	40.8		
Approac	:h	161	2.0	0.161	6.9	LOS A	0.9	6.1	0.35	0.57	42.4		
East: Sn	nith Stre	et											
4	L	18	2.0	0.140	6.8	LOS A	0.7	5.2	0.34	0.59	42.6		
5	Т	83	2.0	0.140	5.9	LOS A	0.7	5.2	0.34	0.50	43.0		
6	R	40	2.0	0.140	9.9	LOS A	0.7	5.2	0.34	0.74	40.8		
Approac	:h	141	2.0	0.140	7.1	LOS A	0.7	5.2	0.34	0.58	42.3		
North: J	ones Str	eet											
7	L	88	2.0	0.207	6.9	LOS A	1.1	8.2	0.37	0.60	42.5		
8	Т	102	2.0	0.207	6.0	LOS A	1.1	8.2	0.37	0.52	42.8		
9	R	19	2.0	0.207	10.0	LOS A	1.1	8.2	0.37	0.75	40.8		
Approac	h	209	2.0	0.207	6.8	LOS A	1.1	8.2	0.37	0.57	42.5		
West: S	mith Stre	et											
10	L	11	2.0	0.129	7.1	LOS A	0.7	4.7	0.39	0.61	42.5		
11	т	99	2.0	0.129	6.1	LOS A	0.7	4.7	0.39	0.53	42.9		
12	R	15	2.0	0.129	10.2	LOS A	0.7	4.7	0.39	0.77	40.7		
Approac	:h	124	2.0	0.129	6.7	LOS A	0.7	4.7	0.39	0.57	42.6		
All Vehic	cles	636	2.0	0.207	6.9	LOS A	1.1	8.2	0.36	0.57	42.4		

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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13S1210200 Bonds Spinning Mills - Existing Conditions.sip 8000056, GTA CONSULTANTS, ENTERPRISE

13S1210200 Jones Street-Dunmore Street Roundabout

Moven	nent Per	formance	- Vehicles								
	Ŧ	Demand	1.15.7	Deg.	Average	Level of	95% Back o	of Queue	Prop.	Effective	Average
MOV ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
Osutha		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Jones Str	eet									
1	L	174	2.0	0.427	11.7	LOS A	2.9	20.9	0.81	0.91	39.0
2	Т	53	2.0	0.427	10.8	LOS A	2.9	20.9	0.81	0.89	39.2
3	R	35	2.0	0.427	14.8	LOS B	2.9	20.9	0.81	0.95	37.5
Approa	ch	261	2.0	0.427	11.9	LOS A	2.9	20.9	0.81	0.91	38.8
East: D	unmore S	Street									
4	L	61	2.0	0.634	9.4	LOS A	6.0	42.8	0.71	0.76	41.2
5	Т	525	2.0	0.634	8.4	LOS A	6.0	42.8	0.71	0.72	41.5
6	R	22	2.0	0.634	12.4	LOS A	6.0	42.8	0.71	0.84	39.4
Approa	ch	608	2.0	0.634	8.7	LOS A	6.0	42.8	0.71	0.73	41.4
North:	lones Stre	eet									
7	L	12	2.0	0.102	8.9	LOS A	0.5	3.8	0.58	0.69	41.0
8	Т	18	2.0	0.102	8.0	LOS A	0.5	3.8	0.58	0.64	41.4
9	R	46	2.0	0.102	12.0	LOS A	0.5	3.8	0.58	0.77	39.2
Approa	ch	76	2.0	0.102	10.6	LOS A	0.5	3.8	0.58	0.73	40.0
West: D	ounmore \$	Street									
10	L	56	2.0	0.415	6.8	LOS A	3.1	22.2	0.41	0.58	42.3
11	т	233	2.0	0.415	5.8	LOS A	3.1	22.2	0.41	0.50	42.6
12	R	182	20	0 415	9.8	LOSA	31	22.2	0.41	0 71	40 7
Approa	ch .	.02 471	2.0	0.415	7.5		3.1	22.2	0.41	0.50	41.8
лрргоа		471	2.0	0.415	7.5	LOGA	5.1	22.2	0.41	0.59	41.0
All Vehi	cles	1416	2.0	0.634	9.0	LOS A	6.0	42.8	0.62	0.72	40.9

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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13S1210200 Jones Street-Dunmore Street Roundabout

Movem	Movement Performanc		- Vehicles								
		Demand	111/	Deg.	Average	Level of	95% Back of	of Queue	Prop.	Effective	Average
NOV ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
South	lonos Str	veh/h	%	V/C	sec		veh	m		per veh	km/h
30utii. 3	101165 31	440	2.0	0 400	0.7		4.4	7.0	0.50	0.74	44.4
1	L -	113	2.0	0.199	8.7	LOSA	1.1	7.8	0.58	0.71	41.4
2	1	19	2.0	0.199	1.1	LOSA	1.1	7.8	0.58	0.65	41.7
3	R	24	2.0	0.199	11.7	LOSA	1.1	7.8	0.58	0.79	39.5
Approac	ch	156	2.0	0.199	9.0	LOS A	1.1	7.8	0.58	0.71	41.1
East: Du	unmore S	Street									
4	L	28	2.0	0.385	7.6	LOS A	2.4	17.4	0.50	0.66	42.2
5	Т	333	2.0	0.385	6.7	LOS A	2.4	17.4	0.50	0.60	42.4
6	R	15	2.0	0.385	10.7	LOS A	2.4	17.4	0.50	0.80	40.5
Approac	ch	376	2.0	0.385	6.9	LOS A	2.4	17.4	0.50	0.61	42.3
North: J	ones Stre	eet									
7	L	16	2.0	0.094	8.9	LOS A	0.5	3.4	0.56	0.69	41.1
8	Т	21	2.0	0.094	7.9	LOS A	0.5	3.4	0.56	0.63	41.5
9	R	34	2.0	0.094	12.0	LOS A	0.5	3.4	0.56	0.78	39.3
Approac	ch	71	2.0	0.094	10.1	LOS A	0.5	3.4	0.56	0.71	40.3
West [.] D	unmore	Street									
10	L	34	2.0	0.362	6.3	LOS A	2.5	18.1	0.26	0.55	42.9
11	т	272	2.0	0.362	5.3	LOSA	2.5	18.1	0.26	0.46	43.4
12	R	152	2.0	0.362	9.0	LOSA	2.5	18.1	0.26	0.72	40.9
Approac		457	2.0	0.362	6.7		2.5	18.1	0.20	0.55	12.5
Appioad	211	4 57	2.0	0.302	0.7	LUGA	2.0	10.1	0.20	0.00	42.0
All Vehic	cles	1059	2.0	0.385	7.4	LOS A	2.5	18.1	0.41	0.61	42.1

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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13S1210200 Bonds Spinning Mills - Existing Conditions.sip 8000056, GTA CONSULTANTS, ENTERPRISE

13S1210200

Dunmore Street-Goodall Street

Signals - Fixed Time Cycle Time = 56 seconds (Optimum Cycle Time - Minimum Delay)

Movem	Movement Performance - Vehicles											
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h	
East: Du	inmore	Street										
5	Т	447	2.0	0.502	9.9	LOS A	8.1	57.4	0.70	0.60	38.7	
6	R	260	2.0	0.573	21.1	LOS B	5.7	40.4	0.91	0.82	33.1	
Approac	h	707	2.0	0.573	14.0	LOS A	8.1	57.4	0.77	0.68	36.5	
North: G	oodall S	Street										
7	L	243	2.0	0.425	15.2	LOS B	3.9	27.5	0.61	0.75	36.6	
9	R	398	2.0	0.761	29.4	LOS C	11.1	79.3	0.97	0.92	29.3	
Approac	h	641	2.0	0.761	24.0	LOS B	11.1	79.3	0.83	0.86	31.7	
West: Du	unmore	Street										
10	L	233	2.0	0.216	10.0	LOS A	2.3	16.6	0.39	0.70	40.3	
11	Т	233	2.0	0.423	18.0	LOS B	5.3	37.9	0.86	0.71	33.4	
Approac	h	465	2.0	0.423	14.0	LOS A	5.3	37.9	0.62	0.71	36.5	
All Vehic	les	1814	2.0	0.761	17.6	LOS B	11.1	79.3	0.76	0.75	34.6	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Mover	Movement Performance - Pedestrians													
MoviD	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective						
	Description	FIOW	Delay	Service	Pedestrian	Distance	Queued	Stop Rate						
		ped/h	sec		ped	m		per ped						
P5	Across N approach	53	22.3	LOS C	0.1	0.1	0.89	0.89						
P7	Across W approach	53	22.3	LOS C	0.1	0.1	0.89	0.89						
All Ped	estrians	106	22.3	LOS C			0.89	0.89						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Processed: Monday, 2 March 2015 1:03:02 PM SIDRA INTERSECTION 5.1.13.2093 Project: P:\13S1200-1299\13S1210200 - Bonds Pendle Hill Additional Modelling\Modelling\150302sid -13S1210200 Bonds Spinning Mills - Existing Conditions.sip 8000056, GTA CONSULTANTS, ENTERPRISE



13S1210200

Dunmore Street-Goodall Street

Signals - Fixed Time Cycle Time = 56 seconds (Optimum Cycle Time - Minimum Delay)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h	
East: Du	inmore	Street										
5	Т	281	2.0	0.315	8.9	LOS A	4.5	32.3	0.62	0.53	39.7	
6	R	201	2.0	0.451	21.0	LOS B	4.2	30.1	0.87	0.79	33.2	
Approac	h	482	2.0	0.451	13.9	LOS A	4.5	32.3	0.73	0.64	36.7	
North: G	oodall S	Street										
7	L	195	2.0	0.340	15.0	LOS B	3.0	21.4	0.59	0.74	36.8	
9	R	209	2.0	0.400	24.4	LOS B	4.8	34.0	0.85	0.79	31.5	
Approac	h	404	2.0	0.400	19.9	LOS B	4.8	34.0	0.73	0.77	33.8	
West: Du	unmore	Street										
10	L	165	2.0	0.157	9.9	LOS A	1.6	11.6	0.37	0.69	40.4	
11	Т	269	2.0	0.482	18.2	LOS B	6.2	44.2	0.87	0.72	33.3	
Approac	h	435	2.0	0.482	15.0	LOS B	6.2	44.2	0.68	0.71	35.7	
All Vehic	les	1321	2.0	0.482	16.1	LOS B	6.2	44.2	0.71	0.70	35.4	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians													
MoviD	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective					
	Description	FIOW	Delay	Service	Pedestrian	Distance	Queuea	Stop Rate					
		ped/h	sec		ped	m		per ped					
P5	Across N approach	53	22.3	LOS C	0.1	0.1	0.89	0.89					
P7	Across W approach	53	22.3	LOS C	0.1	0.1	0.89	0.89					
All Ped	estrians	106	22.3	LOS C			0.89	0.89					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Processed: Monday, 2 March 2015 1:03:08 PM SIDRA INTERSECTION 5.1.13.2093 Project: P:\13S1200-1299\13S1210200 - Bonds Pendle Hill Additional Modelling\Modelling\150302sid -13S1210200 Bonds Spinning Mills - Existing Conditions.sip 8000056, GTA CONSULTANTS, ENTERPRISE



13S1210200

Dunmore Street-Pendle Way

Signals - Fixed Time Cycle Time = 60 seconds (Optimum Cycle Time - Minimum Delay)

Movem	Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h		
South: P	endle \	Nay											
2	Т	277	2.0	0.308	10.8	LOS A	5.1	36.2	0.66	0.56	38.2		
3	R	255	2.0	0.573	22.5	LOS B	6.0	42.7	0.91	0.82	32.4		
Approac	h	532	2.0	0.573	16.4	LOS B	6.0	42.7	0.78	0.68	35.2		
East: Du	inmore	Street											
4	L	420	2.0	0.430	15.7	LOS B	7.5	53.5	0.65	0.78	36.3		
6	R	398	2.0	0.652	25.5	LOS B	10.3	73.2	0.91	0.84	31.0		
Approac	h	818	2.0	0.652	20.5	LOS B	10.3	73.2	0.77	0.81	33.5		
North: P	endle V	Vay											
7	L	199	2.0	0.184	9.7	LOS A	1.9	13.9	0.36	0.69	40.6		
8	Т	188	2.0	0.367	19.8	LOS B	4.6	32.9	0.86	0.70	32.4		
Approac	h	387	2.0	0.367	14.6	LOS B	4.6	32.9	0.60	0.70	36.2		
All Vehic	les	1737	2.0	0.652	17.9	LOS B	10.3	73.2	0.74	0.75	34.6		

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	Movement Performance - Pedestrians													
	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective						
	Description	FIOW	Delay	Service	Pedestrian	Distance	Queuea	Stop Rate						
		ped/h	sec		ped	m		per ped						
P1	Across S approach	53	12.0	LOS B	0.1	0.1	0.63	0.63						
P3	Across E approach	53	24.3	LOS C	0.1	0.1	0.90	0.90						
All Pede	estrians	106	18.2	LOS B			0.77	0.77						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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13S1210200

Dunmore Street-Pendle Way

Signals - Fixed Time Cycle Time = 50 seconds (Optimum Cycle Time - Minimum Delay)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h	
South: P	Pendle V	Vay										
2	Т	303	2.0	0.449	6.5	LOS A	4.2	29.6	0.57	0.48	41.5	
3	R	217	2.0	0.449	16.0	LOS B	4.2	29.6	0.79	0.80	36.2	
Approac	h	520	2.0	0.449	10.4	LOS A	4.2	29.6	0.66	0.61	39.1	
East: Du	inmore	Street										
4	L	255	2.0	0.316	16.5	LOS B	4.1	29.4	0.69	0.77	35.8	
6	R	238	2.0	0.650	28.2	LOS B	5.8	41.4	0.97	0.86	29.8	
Approac	h	493	2.0	0.650	22.1	LOS B	5.8	41.4	0.82	0.81	32.6	
North: P	endle W	Vay										
7	L	226	2.0	0.211	10.4	LOS A	2.3	16.2	0.44	0.71	40.0	
8	Т	222	2.0	0.361	14.5	LOS B	4.3	30.6	0.81	0.67	35.5	
Approac	h	448	2.0	0.361	12.5	LOS A	4.3	30.6	0.62	0.69	37.6	
All Vehic	les	1461	2.0	0.650	15.0	LOS B	5.8	41.4	0.70	0.70	36.2	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians													
MoviD	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective					
	Description	FIOW	Delay	Service	Pedestrian	Distance	Queued	Stop Rate					
		ped/h	sec		ped	m		per ped					
P1	Across S approach	53	14.4	LOS B	0.1	0.1	0.76	0.76					
P3	Across E approach	53	19.4	LOS B	0.1	0.1	0.88	0.88					
All Pede	estrians	106	16.9	LOS B			0.82	0.82					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Processed: Monday, 2 March 2015 1:03:19 PM SIDRA INTERSECTION 5.1.13.2093 Project: P:\13S1200-1299\13S1210200 - Bonds Pendle Hill Additional Modelling\Modelling\150302sid -13S1210200 Bonds Spinning Mills - Existing Conditions.sip 8000056, GTA CONSULTANTS, ENTERPRISE



13S1210200 - Bonds Pendle Hill Additional Modelling Great Western Highway / Jones Street Existing Thursday PM Peak Signals - Fixed Time Cycle Time = 140 seconds (User-Given Phase Times)

Movement Performance - Vehicles											
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: E	Berith Rd	- S									
4	L	43	2.4	0.427	55.2	LOS D	2.4	16.9	0.84	0.73	24.3
5	Т	18	0.0	0.329	60.4	LOS E	4.3	30.7	0.95	0.74	19.4
6	R	51	2.1	0.329	68.1	LOS E	4.3	30.7	0.95	0.77	21.6
Approac	h	112	1.9	0.427	61.9	LOS E	4.3	30.7	0.91	0.75	22.2
East: Gt	Western	ı Hwy - E									
7	L	60	0.0	0.512	22.2	LOS B	21.9	155.7	0.54	1.11	46.0
8	Т	1922	1.8	0.512	12.1	LOS A	22.1	157.3	0.54	0.50	54.3
9	R	115	0.0	0.455	48.5	LOS D	5.6	38.9	0.96	0.78	27.8
Approac	h	2097	1.7	0.512	14.4	LOS A	22.1	157.3	0.57	0.53	51.7
North: Jo	ones St -	N									
10	L	60	0.0	0.306	38.6	LOS C	2.6	18.5	0.69	0.73	29.1
11	Т	29	0.0	0.623	67.3	LOS E	7.1	50.3	1.00	0.81	18.2
12	R	75	1.4	0.623	75.0	LOS F	7.1	50.3	1.00	0.81	20.4
Approac	h	164	0.6	0.623	60.3	LOS E	7.1	50.3	0.89	0.78	22.5
West: G	t Westeri	n Hwy - W									
1	L	44	0.0	0.416	29.5	LOS C	17.0	120.9	0.63	1.05	40.0
2	Т	1271	2.2	0.416	19.4	LOS B	17.2	122.5	0.63	0.56	46.5
3	R	43	2.4	0.414	82.6	LOS F	3.0	21.5	1.00	0.74	19.1
Approac	h	1358	2.2	0.416	21.7	LOS B	17.2	122.5	0.64	0.58	44.6
All Vehic	les	3731	1.8	0.623	20.5	LOS B	22.1	157.3	0.62	0.56	44.9

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped						
P3	Across S approach	53	11.2	LOS B	0.1	0.1	0.40	0.40						
P7	Across N approach	53	19.0	LOS B	0.1	0.1	0.52	0.52						
P1	Across W approach	53	60.4	LOS F	0.2	0.2	0.93	0.93						
All Ped	estrians	159	30.2	LOS D			0.62	0.62						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Processed: Tuesday, 3 March 2015 3:11:01 PM SIDRA INTERSECTION 5.1.13.2093 Project: \\gta.com.au\projectfiles\ProjectFilesSyd\13S1200-1299\13S1210200 - Bonds Pendle Hill Additional Modelling\Modelling\150304sid_13S1210200 Bonds,Pendle Hill - Existing Conditions.sip 8000056, GTA CONSULTANTS, ENTERPRISE



13S1210200 - Bonds Pendle Hill Additional Modelling Great Western Highway / Jones Street Existing Saturday Peak Signals - Fixed Time Cycle Time = 130 seconds (User-Given Phase Times)

Movement Performance - Vehicles												
May ID	T	Demand		Deg.	Average	Level of	95% Back (of Queue	Prop.	Effective	Average	
	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed	
South: D	orith Dd	ven/n	%	V/C	sec	_	ven	m		per ven	km/n	
		- 3	17	0 5 9 7	52.0		2.2	22.0	0.96	0.76	22.0	
4	L -	03	1.7	0.007	52.9		3.3	23.0	0.00	0.76	22.0	
5	-	15	0.0	0.468	61.3	LOSE	4.6	32.9	0.98	0.77	21.4	
6	R	59	3.6	0.468	67.5	LOSE	4.6	32.9	0.98	0.78	19.2	
Approac	h	137	2.3	0.587	60.1	LOS E	4.6	32.9	0.93	0.77	20.7	
East: Gt	Westerr	n Hwy - E										
7	L	75	1.4	0.282	23.8	LOS B	12.9	92.7	0.63	1.05	45.0	
8	Т	999	3.5	0.282	16.7	LOS B	14.1	101.8	0.67	0.66	48.2	
9	R	81	3.9	0.343	48.9	LOS D	3.9	28.3	0.95	0.76	27.7	
Approac	h	1155	3.4	0.343	19.5	LOS B	14.1	101.8	0.69	0.69	45.7	
North: Jo	ones St -	· N										
10	L	66	0.0	0.333	39.7	LOS C	2.9	20.1	0.73	0.74	28.7	
11	Т	29	0.0	0.587	67.5	LOS E	4.3	30.9	1.00	0.78	18.2	
12	R	36	5.9	0.587	75.4	LOS F	4.3	30.9	1.00	0.78	20.4	
Approac	h	132	1.6	0.587	55.6	LOS D	4.3	30.9	0.86	0.76	23.4	
West: G	t Wester	n Hwy - W										
1	L	27	0.0	0.382	25.9	LOS B	14.2	101.0	0.58	1.07	42.8	
2	Т	1242	2.0	0.382	15.7	LOS B	14.3	101.9	0.58	0.52	50.1	
3	R	61	1.7	0.393	73.2	LOS F	3.8	27.1	0.99	0.76	20.9	
Approac	h	1331	2.0	0.393	18.6	LOS B	14.3	101.9	0.60	0.54	47.4	
All Vehic	les	2754	2.6	0.587	22.8	LOS B	14.3	101.9	0.67	0.62	42.2	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped				
P3	Across S approach	53	10.8	LOS B	0.1	0.1	0.41	0.41				
P7	Across N approach	53	16.3	LOS B	0.1	0.1	0.50	0.50				
P1	Across W approach	53	58.2	LOS E	0.2	0.2	0.95	0.95				
All Pedestrians		159	28.4	LOS C			0.62	0.62				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Processed: Tuesday, 3 March 2015 3:11:03 PM SIDRA INTERSECTION 5.1.13.2093 Project: \\gta.com.au\projectfiles\ProjectFilesSyd\13S1200-1299\13S1210200 - Bonds Pendle Hill Additional Modelling\Modelling\150304sid_13S1210200 Bonds,Pendle Hill - Existing Conditions.sip 8000056, GTA CONSULTANTS, ENTERPRISE



13S1210200 - Bonds Pendle Hill Additional Modelling Cumberland Highway / Dunmore Street Existing Thursday PM Peak Signals - Fixed Time Cycle Time = 140 seconds (User-Given Phase Times)

Movem	ent Per	formance - \	/ehicles								
Mov ID	Turn	Demand Flow	HV	Deg. Satn	Average Delay	Level of Service	95% Back o Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: C	Cumberla	ind Hwy - S									
1	L	49	0.0	0.697	43.7	LOS D	33.8	241.8	0.82	0.98	30.1
2	Т	1946	2.9	0.697	36.0	LOS C	34.0	243.6	0.82	0.75	31.4
3	R	42	0.0	0.244	73.7	LOS F	2.7	19.2	0.97	0.74	20.0
Approac	h	2038	2.8	0.697	36.9	LOS C	34.0	243.6	0.82	0.75	31.0
East: Du	inmore S	St - E									
4	L	62	0.0	0.625	38.5	LOS C	13.3	93.8	0.73	0.83	28.4
5	Т	447	1.2	0.834	35.8	LOS C	26.5	186.8	0.86	0.76	25.6
6	R	256	0.4	0.834	48.2	LOS D	26.5	186.8	1.00	0.91	25.4
Approac	h	765	0.8	0.834	40.2	LOS C	26.5	186.8	0.90	0.82	25.7
North: C	umberla	nd Hwy - N									
7	L	295	1.4	0.487	22.4	LOS B	7.2	50.8	0.39	0.75	39.4
8	Т	2145	4.4	0.752	37.0	LOS C	38.0	276.3	0.85	0.78	31.0
9	R	143	3.7	0.586	36.8	LOS C	5.3	38.2	0.95	0.82	31.0
Approac	h	2583	4.0	0.752	35.3	LOS C	38.0	276.3	0.81	0.78	31.7
West: D	unmore	St - W									
10	L	88	0.0	0.530	61.5	LOS E	11.1	78.1	0.94	0.82	22.1
11	Т	226	1.9	0.530	53.8	LOS D	11.1	78.1	0.94	0.78	20.9
12	R	41	5.1	0.530	61.4	LOS E	10.7	76.8	0.94	0.82	22.3
Approac	:h	356	1.8	0.530	56.6	LOS E	11.1	78.1	0.94	0.79	21.3
All Vehic	cles	5742	3.0	0.834	37.8	LOS C	38.0	276.3	0.83	0.78	29.7

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Mover	Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped						
P1	Across S approach	53	39.4	LOS D	0.2	0.2	0.75	0.75						
P3	Across E approach	53	32.9	LOS D	0.1	0.1	0.69	0.69						
P5	Across N approach	53	58.5	LOS E	0.2	0.2	0.91	0.91						
P7	Across W approach	53	32.9	LOS D	0.1	0.1	0.69	0.69						
All Ped	estrians	212	40.9	LOS E			0.76	0.76						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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13S1210200 - Bonds Pendle Hill Additional Modelling Cumberland Highway / Dunmore Street Existing Saturday Peak Signals - Fixed Time Cycle Time = 130 seconds (User-Given Phase Times)

Mover	nent Per	formance	- Vehicles								
	-	Demand	111/	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
Ven/n		veh/h	%	V/C	sec		veh	m		per veh	km/h
300011. 0		iliu riwy - 3	0.0	0 711	42.6		24.6	176.0	0.04	0.07	20.2
1	L -	23	0.0	0.711	43.0	LOSD	24.0	170.3	0.84	0.97	30.2
2	1	14/8	3.0	0.711	35.9	LUSC	24.7	177.4	0.84	0.76	31.4
3	R	68	1.5	0.538	/5.0	LOSF	4.4	31.4	1.00	0.76	19.8
Approad	ch	1569	2.9	0.711	37.7	LOS C	24.7	177.4	0.85	0.76	30.6
East: Du	unmore S	St - E									
4	L	54	3.9	0.519	33.6	LOS C	6.5	46.3	0.68	0.81	30.2
5	Т	245	0.4	0.865	34.7	LOS C	22.0	155.2	0.86	0.78	25.9
6	R	265	0.8	0.865	49.3	LOS D	22.0	155.2	1.00	0.97	25.0
Approad	ch	564	0.9	0.865	41.5	LOS C	22.0	155.2	0.91	0.87	25.8
North: C	Cumberla	nd Hwy - N									
7	L	299	1.8	0.638	26.9	LOS B	10.4	74.1	0.60	0.78	36.3
8	Т	1540	2.7	0.725	36.1	LOS C	25.5	182.9	0.85	0.77	31.4
9	R	111	0.0	0.545	36.8	LOS C	4.1	28.5	0.94	0.80	30.9
Approac	ch	1949	2.4	0.725	34.7	LOS C	25.5	182.9	0.82	0.77	32.0
West: D	unmore	St - W									
10	L	94	0.0	0.520	56.0	LOS D	10.8	75.8	0.93	0.82	23.3
11	Т	206	0.5	0.520	48.4	LOS D	10.8	75.8	0.93	0.77	22.0
12	R	72	1.5	0.520	55.9	LOS D	10.1	71.1	0.93	0.82	23.4
Approad	ch	372	0.6	0.520	51.8	LOS D	10.8	75.8	0.93	0.79	22.6
All Vehi	cles	4455	2.2	0.865	38.1	LOS C	25.5	182.9	0.85	0.78	29.6

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Mover	Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped						
P1	Across S approach	53	34.7	LOS D	0.1	0.1	0.73	0.73						
P3	Across E approach	53	32.6	LOS D	0.1	0.1	0.71	0.71						
P5	Across N approach	53	53.6	LOS E	0.2	0.2	0.91	0.91						
P7	Across W approach	53	32.6	LOS D	0.1	0.1	0.71	0.71						
All Ped	estrians	212	38.3	LOS D			0.76	0.76						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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13S1210200 - Bonds Pendle Hill Additional Modelling Cumberland Highway / Smith Street Existing Thursday PM Peak Signals - Fixed Time Cycle Time = 140 seconds (User-Given Phase Times)

Movem	ent Pe	rformance - ^v	Vehicles								
	-	Demand	1.15.7	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
MOV ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
V		veh/h	%	V/C	sec	_	veh	m	_	per veh	km/h
South: C	Jumberia	and Hwy - S									
1	L	111	1.0	0.595	17.6	LOS B	17.2	123.2	0.40	0.96	41.5
2	Т	2024	3.0	0.595	8.5	LOS A	17.2	123.2	0.36	0.33	47.2
3	R	55	0.0	0.375	76.5	LOS F	3.7	25.8	0.99	0.75	19.3
Approac	h	2189	2.8	0.595	10.7	LOS A	17.2	123.2	0.38	0.38	45.2
East: Sn	nith St -	E									
4	L	57	0.0	0.429	62.4	LOS E	5.5	38.9	0.91	0.79	22.4
5	Т	237	1.8	0.859	69.3	LOS E	17.0	120.7	0.99	0.94	19.8
6	R	28	0.0	0.859	79.9	LOS F	17.0	120.7	1.00	0.98	19.4
Approac	:h	322	1.3	0.859	69.0	LOS E	17.0	120.7	0.97	0.92	20.2
North: C	umberla	and Hwy - N									
7	L	42	2.5	0.591	16.9	LOS B	15.8	114.4	0.37	1.00	42.2
8	Т	2065	4.4	0.591	8.3	LOS A	15.8	114.4	0.35	0.32	47.5
9	R	47	2.2	0.330	76.3	LOS F	3.2	22.7	0.98	0.75	19.4
Approac	:h	2155	4.3	0.591	9.9	LOS A	15.8	114.4	0.37	0.35	45.9
West: Si	mith St -	W									
10	L	31	0.0	0.333	63.4	LOS E	4.2	30.0	0.91	0.78	22.3
11	Т	111	4.8	0.666	64.5	LOS E	7.4	53.0	0.97	0.78	20.6
12	R	35	0.0	0.666	77.6	LOS F	7.4	53.0	1.00	0.82	19.6
Approac	:h	176	3.0	0.666	66.9	LOS E	7.4	53.0	0.96	0.79	20.7
All Vehic	cles	4842	3.4	0.859	16.3	LOS B	17.2	123.2	0.44	0.41	40.4

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Mover	Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped						
P1	Across S approach	53	59.4	LOS E	0.2	0.2	0.92	0.92						
P3	Across E approach	53	13.7	LOS B	0.1	0.1	0.44	0.44						
P5	Across N approach	53	59.4	LOS E	0.2	0.2	0.92	0.92						
P7	Across W approach	53	13.7	LOS B	0.1	0.1	0.44	0.44						
All Ped	estrians	212	36.6	LOS D			0.68	0.68						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.



13S1210200 - Bonds Pendle Hill Additional Modelling Cumberland Highway / Smith Street Existing Saturday Peak Signals - Fixed Time Cycle Time = 130 seconds (User-Given Phase Times)

Movem	ient Pe	rformance - '	Vehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back (Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: Cumberland Hwy - S		and Hwy - S								· ·	
1	L	113	0.9	0.462	19.5	LOS B	17.6	125.5	0.53	0.93	40.4
2	Т	1639	2.8	0.462	11.3	LOS A	17.7	126.8	0.53	0.48	43.9
3	R	55	3.8	0.394	72.5	LOS F	3.5	25.0	0.99	0.75	20.1
Approac	h	1806	2.7	0.462	13.7	LOS A	17.7	126.8	0.54	0.51	42.2
East: Sr	nith St -	E									
4	L	39	2.7	0.156	48.6	LOS D	1.9	13.5	0.80	0.73	25.7
5	Т	85	0.0	0.541	61.2	LOS E	6.5	45.8	0.99	0.78	21.4
6	R	20	0.0	0.541	69.1	LOS E	6.5	45.8	0.99	0.79	21.4
Approad	h	144	0.7	0.541	58.9	LOS E	6.5	45.8	0.94	0.77	22.4
North: C	umberla	and Hwy - N									
7	L	41	0.0	0.430	19.1	LOS B	16.0	114.3	0.51	0.97	40.8
8	Т	1593	2.9	0.430	11.0	LOS A	16.0	114.7	0.51	0.46	44.4
9	R	55	0.0	0.383	72.2	LOS F	3.4	24.1	0.99	0.75	20.1
Approac	h	1688	2.7	0.430	13.1	LOS A	16.0	114.7	0.53	0.48	42.6
West: S	mith St -	W									
10	L	24	0.0	0.181	60.8	LOS E	2.3	16.0	0.91	0.75	22.8
11	Т	80	0.0	0.635	62.1	LOS E	6.4	45.2	0.98	0.78	21.0
12	R	36	2.9	0.635	72.7	LOS F	6.4	45.2	1.00	0.81	20.5
Approac	h	140	0.8	0.635	64.6	LOS E	6.4	45.2	0.97	0.78	21.2
All Vehic	cles	3779	2.6	0.635	17.0	LOS B	17.7	126.8	0.57	0.52	39.6

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped						
P1	Across S approach	53	59.1	LOS E	0.2	0.2	0.95	0.95						
P3	Across E approach	53	11.2	LOS B	0.1	0.1	0.42	0.42						
P5	Across N approach	53	59.1	LOS E	0.2	0.2	0.95	0.95						
P7	Across W approach	53	11.2	LOS B	0.1	0.1	0.42	0.42						
All Pedestrians		212	35.2	LOS D			0.68	0.68						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.


13S1210200 - Bonds Pendle Hill Additional Modelling Gilba Road / Pendle Way Existing Thursday PM Peak Giveway / Yield (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h	
South: F	Pendle \	Nay - S										
1	L	434	1.0	0.235	8.2	LOS A	0.0	0.0	0.00	0.67	49.0	
2	Т	192	0.5	0.592	15.9	LOS B	2.5	17.3	0.61	0.97	41.7	
Approac	h	625	0.8	0.592	10.6	LOS A	2.5	17.3	0.19	0.76	46.5	
North: P	endle V	Vay - N										
8	Т	146	4.3	0.855	40.1	LOS C	5.9	42.8	0.64	1.41	28.5	
9	R	51	2.1	0.855	41.4	LOS C	5.9	42.8	0.64	1.29	28.2	
Approac	h	197	3.7	0.855	40.4	LOS C	5.9	42.8	0.64	1.38	28.4	
West: G	ilba Rd	- W										
10	L	163	0.6	0.214	8.2	LOS A	0.0	0.0	0.00	0.66	49.0	
12	R	231	1.4	0.214	8.3	LOS A	0.0	0.0	0.00	0.67	48.9	
Approac	h	394	1.1	0.214	8.2	NA	0.0	0.0	0.00	0.67	48.9	
All Vehic	les	1216	1.4	0.855	14.6	NA	5.9	42.8	0.20	0.83	42.8	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

Processed: Tuesday, 3 March 2015 3:28:41 PM SIDRA INTERSECTION 5.1.13.2093

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13S1210200 - Bonds Pendle Hill Additional Modelling Gilba Road / Pendle Way **Existing Saturday Peak** Giveway / Yield (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h	
South: P	Pendle V	Vay - S										
1	L	281	2.2	0.154	8.3	LOS A	0.0	0.0	0.00	0.67	49.0	
2	Т	196	0.0	0.590	16.7	LOS B	2.6	18.4	0.64	1.00	41.1	
Approac	h	477	1.3	0.590	11.7	LOS A	2.6	18.4	0.26	0.80	45.4	
North: P	endle V	Vay - N										
8	Т	145	3.6	0.860	38.1	LOS C	6.4	46.3	0.68	1.46	29.2	
9	R	73	1.4	0.860	39.4	LOS C	6.4	46.3	0.68	1.35	28.9	
Approac	h	218	2.9	0.860	38.5	LOS C	6.4	46.3	0.68	1.42	29.1	
West: Gi	ilba Rd	- W										
10	L	171	1.2	0.233	8.2	LOS A	0.0	0.0	0.00	0.66	49.0	
12	R	260	0.4	0.233	8.2	LOS A	0.0	0.0	0.00	0.67	48.9	
Approac	h	431	0.7	0.233	8.2	NA	0.0	0.0	0.00	0.67	48.9	
All Vehic	les	1125	1.4	0.860	15.6	NA	6.4	46.3	0.24	0.87	42.0	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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13S1210200 - Bonds Pendle Hill Additional Modelling Goodall Street / Wentworth Avenue Existing Thursday PM Peak Signals - Fixed Time Cycle Time = 90 seconds (User-Given Phase Times)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h	
South: G	Goodall	St - S										
1	L	305	1.7	0.518	15.0	LOS B	5.8	41.3	0.47	0.74	37.9	
3	R	246	0.9	0.706	46.3	LOS D	10.7	75.5	0.99	0.86	24.5	
Approac	h	552	1.3	0.706	29.0	LOS C	10.7	75.5	0.70	0.80	30.5	
East: We	entworth	n Ave - E										
4	L	312	0.3	0.734	26.7	LOS B	9.0	62.9	0.69	0.82	33.5	
5	Т	324	2.9	0.605	33.6	LOS C	12.9	92.3	0.95	0.80	29.8	
Approac	:h	636	1.7	0.734	30.2	LOS C	12.9	92.3	0.82	0.81	31.4	
West: W	/entwort	h Ave - W										
11	Т	335	2.8	0.459	5.8	LOS A	5.4	38.9	0.40	0.34	50.1	
12	R	457	2.3	0.552	31.2	LOS C	15.8	112.4	0.84	0.84	31.2	
Approac	h	792	2.5	0.552	20.5	LOS B	15.8	112.4	0.66	0.63	37.4	
All Vehic	les	1979	1.9	0.734	26.0	LOS B	15.8	112.4	0.72	0.73	33.2	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	Movement Performance - Pedestrians												
	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective					
	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate					
		ped/h	sec		ped	m		per ped					
P7	Across W approach	53	36.5	LOS D	0.1	0.1	0.90	0.90					
All Pedestrians 53 36.5 LOS D 0.90								0.90					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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13S1210200 - Bonds Pendle Hill Additional Modelling Goodall Street / Wentworth Avenue **Existing Saturday Peak** Signals - Fixed Time Cycle Time = 90 seconds (User-Given Phase Times)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back c Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h	
South: C	Goodall	St - S										
1	L	305	1.7	0.518	16.0	LOS B	5.8	41.3	0.47	0.75	41.7	
3	R	246	0.9	0.858	56.9	LOS E	12.2	86.1	1.00	0.96	23.4	
Approac	h	552	1.3	0.858	34.2	LOS C	12.2	86.1	0.71	0.85	30.9	
East: We	entwort	h Ave - E										
4	L	312	0.3	0.762	30.9	LOS C	9.7	68.3	0.73	0.84	32.4	
5	Т	324	2.9	0.559	33.0	LOS C	12.7	90.8	0.94	0.79	30.0	
Approac	h	636	1.7	0.762	32.0	LOS C	12.7	90.8	0.83	0.81	31.1	
West: W	entwor	th Ave - W										
11	Т	335	2.8	0.414	4.6	LOS A	4.8	34.3	0.36	0.30	51.7	
12	R	457	2.3	0.468	29.0	LOS C	14.5	103.7	0.78	0.83	33.4	
Approac	h	792	2.5	0.468	18.6	LOS B	14.5	103.7	0.60	0.60	39.3	
All Vehic	les	1979	1.9	0.858	27.3	LOS B	14.5	103.7	0.71	0.74	33.9	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	Movement Performance - Pedestrians												
May ID	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective					
	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate					
		ped/h	sec		ped	m		per ped					
P7	Across W approach	53	39.2	LOS D	0.1	0.1	0.93	0.93					
All Pede	estrians	53	39.2	LOS D			0.93	0.93					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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13S1210200 - Bonds Pendle Hill Additional Modelling Great Western Highway / Pendle Way Existing Thursday PM Peak Signals - Fixed Time Cycle Time = 140 seconds (User-Given Phase Times)

Movement Performance - Vehicles											
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: E	ttalong F	Rd - S					Volt				TXTT 0 TT
4	L	105	1.0	0.513	53.1	LOS D	5.7	39.9	0.84	0.76	24.4
5	Т	207	1.0	0.933	75.2	LOS F	14.4	101.3	1.00	0.90	18.8
6	R	104	0.0	0.933	93.6	LOS F	14.4	101.3	1.00	1.04	17.0
Approac	:h	417	0.8	0.933	74.2	LOS F	14.4	101.3	0.96	0.90	19.4
East: Gt	Westerr	ı Hwy - E									
7	L	221	0.5	0.614	40.4	LOS C	26.7	189.4	0.77	0.94	29.1
8	Т	1553	2.2	0.614	29.8	LOS C	26.8	191.1	0.73	0.67	31.6
9	R	140	1.5	0.897	85.2	LOS F	10.4	73.4	0.99	0.95	18.0
Approac	h	1914	2.0	0.897	35.1	LOS C	26.8	191.1	0.76	0.72	29.6
North: P	endle W	ay - N									
10	L	77	1.4	0.629	56.7	LOS E	14.1	99.7	0.94	0.85	24.1
11	Т	267	1.6	0.786	52.1	LOS D	24.9	178.9	0.96	0.83	23.3
12	R	278	3.8	0.786	66.6	LOS E	24.9	178.9	1.00	0.88	21.5
Approac	h	622	2.5	0.786	59.2	LOS E	24.9	178.9	0.98	0.86	22.5
West: G	t Wester	n Hwy - W									
1	L	113	7.5	0.480	39.6	LOS C	18.2	131.9	0.78	0.88	29.7
2	Т	1035	2.6	0.480	31.2	LOS C	18.9	135.4	0.78	0.68	30.9
3	R	114	0.9	0.539	72.7	LOS F	7.5	53.1	0.99	0.79	20.0
Approac	h	1261	2.9	0.539	35.7	LOS C	18.9	135.4	0.80	0.71	29.4
All Vehic	cles	4214	2.2	0.933	42.7	LOS D	26.8	191.1	0.82	0.75	26.9

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped					
P3	Across S approach	53	31.6	LOS D	0.1	0.1	0.67	0.67					
P5	Across E approach	53	59.4	LOS E	0.2	0.2	0.92	0.92					
P7	Across N approach	53	29.6	LOS C	0.1	0.1	0.65	0.65					
All Pede	estrians	159	40.2	LOS E			0.75	0.75					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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13S1210200 - Bonds Pendle Hill Additional Modelling Great Western Highway / Pendle Way Existing Saturday Peak Signals - Fixed Time Cycle Time = 130 seconds (User-Given Phase Times)

Movement Performance - Vehicles													
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h		
South: E	ttalong l	Rd - S											
4	L	65	1.6	0.279	41.8	LOS C	2.9	20.6	0.74	0.74	27.9		
5	Т	165	0.6	0.807	59.9	LOS E	13.1	92.9	0.98	0.82	21.7		
6	R	138	1.5	0.807	73.0	LOS F	13.1	92.9	1.00	0.92	20.2		
Approac	h	368	1.1	0.807	61.6	LOS E	13.1	92.9	0.94	0.84	21.9		
East: Gt	Westerr	n Hwy - E											
7	L	144	0.7	0.397	36.8	LOS C	12.3	88.0	0.78	0.89	30.5		
8	Т	787	3.9	0.397	30.9	LOS C	14.4	104.0	0.78	0.68	31.0		
9	R	165	3.2	0.985	67.8	LOS E	10.2	73.4	0.99	0.82	21.0		
Approac	h	1097	3.4	0.985	37.3	LOS C	14.4	104.0	0.81	0.73	28.9		
North: P	endle W	'ay - N											
10	L	116	3.6	0.560	41.0	LOS C	7.8	55.8	0.96	0.81	28.8		
11	Т	164	0.6	0.701	46.5	LOS D	14.9	105.9	0.98	0.82	24.5		
12	R	147	2.1	0.701	64.1	LOS E	14.9	105.9	1.00	0.85	22.1		
Approac	h	427	2.0	0.701	51.1	LOS D	14.9	105.9	0.98	0.83	24.6		
West: G	t Wester	n Hwy - W											
1	L	109	1.9	0.530	41.9	LOS C	18.2	129.9	0.83	0.88	28.8		
2	Т	1031	2.1	0.530	33.6	LOS C	18.8	133.9	0.83	0.73	29.8		
3	R	89	1.2	0.409	63.7	LOS E	5.2	37.0	0.95	0.78	21.9		
Approac	h	1229	2.1	0.530	36.6	LOS C	18.8	133.9	0.84	0.74	28.9		
All Vehic	les	3122	2.4	0.985	41.7	LOS C	18.8	133.9	0.86	0.76	27.2		

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped						
P3	Across S approach	53	34.0	LOS D	0.1	0.1	0.72	0.72						
P5	Across E approach	53	59.1	LOS E	0.2	0.2	0.95	0.95						
P7	Across N approach	53	31.9	LOS D	0.1	0.1	0.70	0.70						
All Pede	estrians	159	41.7	LOS E			0.79	0.79						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Attachment 3B – SIDRA Output (2027 Future Base Thursday PM & Saturday Midday)

Intersection	Peak	Leg	Degree of Saturation (DOS)	Average Delay (sec)	95 th Percentile (m)	Level of Service (LOS)
		South	0.17	8	8	А
		East	0.02	9	1	А
	PIVI	North	0.17	7	0	А
Jones Street/		All	0.17	1	8	А
Rogers Street		South	0.11	8	5	А
	Sat	East	0.02	8	1	А
	ડ્યા	North	0.13	7	0	А
		All	0.13	1	5	А
		South	0.16	8	8	А
	DNA	East	0.04	9	1	А
	PIVI	North	0.19	7	0	А
Jones Street/		All	0.19	1	8	А
Oatlands Street		South	0.11	8	5	А
	Cat	East	0.02	8	1	А
	Sat	North	0.13	7	0	А
		All	0.13	1	5	А
		South	0.32	12	14	А
		East	0.39	11	19	А
	PM	North	0.31	10	14	А
		West	0.19	11	7	А
Jones Street/		All	0.39	8	19	А
Smith Street		South	0.20	10	8	А
		East	0.17	10	7	А
	Sat	North	0.26	10	11	А
		West	0.16	10	6	А
		All	0.26	7	11	А
		South	0.62	21	40	В
		East	0.80	17	86	В
	PM	North	0.14	13	5	А
		West	0.51	10	31	А
Jones Street/		All	0.80	12	86	В
Jones Street/ Dunmore Street		South	0.26	13	11	А
		East	0.48	11	24	А
	Sat	North	0.12	13	5	А
		West	0.44	10	24	А
		All	0.48	8	24	A



Intersection	Peak	Leg	Degree of Saturation (DOS)	Average Delay (sec)	95 th Percentile (m)	Level of Service (LOS)
		East	0.80	20	83	В
		North	0.78	24	101	В
	PM	West	0.54	15	51	В
Goodall Street/		All	0.80	20	101	В
Dunmore Street		East	0.58	15	32	В
		North	0.48	20	34	В
	Sat	West	0.59	16	44	В
		All	0.59	17	44	В
		South	0.73	19	59	В
	DI 4	East	0.78	23	101	В
	PM	North	0.44	15	40	В
Pendle Way/		All	0.78	20	101	В
Dunmore Street		South	0.56	11	38	В
		East	0.78	24	54	В
	Sat	North	0.43	13	38	В
		All	0.78	16	54	В
		South	0.50	62	37	E
		East	0.57	15	188	В
	PM	North	0.77	63	62	E
		West	0.48	23	142	В
Great Western		All	0.77	22	188	В
Street		South	0.69	63	40	E
		East	0.41	20	116	В
	Sat	North	0.75	58	38	E
		West	0.46	19	118	В
		All	0.75	24	118	В
		South	0.78	39	292	С
		East	1.01	69	315	E
	PM	North	0.84	38	337	С
		West	0.64	58	96	E
Cumberland		All	1.01	44	337	D
Dunmore Street		South	0.80	40	216	С
		East	1.07	101	305	F
	Sat	North	0.81	38	225	С
		West	0.62	53	92	D
		All	1.07	49	305	D



Intersection	Peak	Leg	Degree of Saturation (DOS)	Average Delay (sec)	95 th Percentile (m)	Level of Service (LOS)
		South	0.67	11	156	А
		East	1.05	122	205	F
	PM	North	0.66	11	144	А
		West	0.92	76	63	F
Cumberland		All	1.05	21	205	В
Street		South	0.52	14	150	А
		East	0.66	60	55	E
	Sat	North	0.48	14	135	А
		West	0.80	68	55	E
		All	0.80	18	150	В
		South	0.41	14	16	А
	514	North	0.59	24	30	В
	PM	West	0.25	8	0	А
Pendle Way /		All	0.59	11	30	А
Gilba Road		South	0.48	16	19	В
		North	0.69	27	37	В
	Sat	West	0.27	8	0	А
		All	0.69	13	37	А
		South	0.83	32	98	С
	DNA	East	0.87	35	113	С
	PM	West	0.65	21	139	В
Goodall Street		All	0.87	29	139	С
/wentworth Avenue		South	0.92	37	100	С
		East	0.62	30	102	С
	Sat	West	0.47	16	81	В
		All	0.92	27	102	В
		South	1.10	116	176	F
		East	1.00	36	233	С
	PM	North	0.93	70	248	E
		West	0.63	37	157	С
Cumberland		All	1.10	49	248	D
Cumberland Highway / Pendle Way		South	0.95	73	130	F
		East	1.00	37	124	С
	Sat	North	0.82	55	134	D
		West	0.60	38	155	С
		All	1.00	44	155	D

13S1210200 Jones Street-Rogers Street Giveway / Yield (Two-Way)

Movem	ient Pe	rformance - '	Vehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: J	lones St	treet									
2	Т	304	2.0	0.168	1.3	LOS A	1.2	8.4	0.46	0.00	44.6
3	R	14	2.0	0.168	8.1	LOS A	1.2	8.4	0.46	0.88	43.0
Approac	ch	318	2.0	0.168	1.6	NA	1.2	8.4	0.46	0.04	44.6
East: Ro	ogers St	treet									
4	L	14	2.0	0.022	8.2	LOS A	0.1	0.6	0.41	0.60	41.7
6	R	7	2.0	0.022	8.6	LOS A	0.1	0.6	0.41	0.73	41.5
Approac	ch	21	2.0	0.022	8.4	LOS A	0.1	0.6	0.41	0.64	41.6
North: J	ones St	reet									
7	L	7	2.0	0.169	6.5	LOS A	0.0	0.0	0.00	0.91	43.3
8	Т	318	2.0	0.169	0.0	LOS A	0.0	0.0	0.00	0.00	50.0
Approac	h	325	2.0	0.169	0.1	NA	0.0	0.0	0.00	0.02	49.8
All Vehic	cles	664	2.0	0.169	1.1	NA	1.2	8.4	0.23	0.05	46.9

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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13S1210200 Jones Street-Rogers Street Giveway / Yield (Two-Way)

Movem	ient Pe	rformance -	Vehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: J	lones St	reet									
2	Т	194	2.0	0.105	0.9	LOS A	0.7	4.7	0.38	0.00	45.5
3	R	6	2.0	0.105	7.7	LOS A	0.7	4.7	0.38	0.87	43.0
Approac	ch	200	2.0	0.105	1.1	NA	0.7	4.7	0.38	0.03	45.4
East: Ro	ogers Sti	reet									
4	L	14	2.0	0.019	7.6	LOS A	0.1	0.5	0.34	0.57	42.0
6	R	7	2.0	0.019	8.0	LOS A	0.1	0.5	0.34	0.68	42.0
Approac	;h	21	2.0	0.019	7.7	LOS A	0.1	0.5	0.34	0.61	42.0
North: Jo	ones Str	eet									
7	L	17	2.0	0.132	6.5	LOS A	0.0	0.0	0.00	0.89	43.3
8	T	237	2.0	0.132	0.0	LOS A	0.0	0.0	0.00	0.00	50.0
Approac	;h	254	2.0	0.132	0.4	NA	0.0	0.0	0.00	0.06	49.5
All Vehic	cles	475	2.0	0.132	1.0	NA	0.7	4.7	0.17	0.07	47.3

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

Processed: Tuesday, 3 March 2015 5:03:12 PM SIDRA INTERSECTION 5.1.13.2093

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Project: P:\13S1200-1299\13S1210200 - Bonds Pendle Hill Additional Modelling\Modelling\150302sid -

13S1210200 Jones Street-Oatlands Street Giveway / Yield (Two-Way)

Movem	ient Pe	rformance - \	/ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: J	ones St	treet									
2	Т	277	2.0	0.159	1.5	LOS A	1.1	7.9	0.47	0.00	44.4
3	R	19	2.0	0.159	8.3	LOS A	1.1	7.9	0.47	0.88	43.0
Approac	h	296	2.0	0.159	1.9	NA	1.1	7.9	0.47	0.06	44.3
East: Oa	atlands	Street									
4	L	29	2.0	0.042	8.2	LOS A	0.2	1.1	0.42	0.62	41.7
6	R	12	2.0	0.042	8.6	LOS A	0.2	1.1	0.42	0.76	41.5
Approac	h	41	2.0	0.042	8.3	LOS A	0.2	1.1	0.42	0.66	41.7
North: J	ones St	reet									
7	L	15	2.0	0.186	6.5	LOS A	0.0	0.0	0.00	0.90	43.3
8	Т	342	2.0	0.186	0.0	LOS A	0.0	0.0	0.00	0.00	50.0
Approac	h	357	2.0	0.186	0.3	NA	0.0	0.0	0.00	0.04	49.7
All Vehic	cles	694	2.0	0.186	1.4	NA	1.1	7.9	0.23	0.08	46.7

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Project: P:\13S1200-1299\13S1210200 - Bonds Pendle Hill Additional Modelling\Modelling\150302sid -

13S1210200 Jones Street-Oatlands Street Giveway / Yield (Two-Way)

Movem	ovement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h		
South: J	ones S	street											
2	Т	185	2.0	0.113	0.9	LOS A	0.7	4.9	0.37	0.00	45.4		
3	R	24	2.0	0.113	7.7	LOS A	0.7	4.9	0.37	0.84	42.9		
Approac	h	209	2.0	0.113	1.7	NA	0.7	4.9	0.37	0.10	45.1		
East: Oa	atland S	Street											
4	L	17	2.0	0.020	7.5	LOS A	0.1	0.5	0.34	0.58	42.1		
6	R	6	2.0	0.020	7.9	LOS A	0.1	0.5	0.34	0.69	42.0		
Approac	h	23	2.0	0.020	7.6	LOS A	0.1	0.5	0.34	0.61	42.0		
North: J	ones S	treet											
7	L	11	2.0	0.130	6.5	LOS A	0.0	0.0	0.00	0.90	43.3		
8	Т	240	2.0	0.130	0.0	LOS A	0.0	0.0	0.00	0.00	50.0		
Approac	h	251	2.0	0.130	0.3	NA	0.0	0.0	0.00	0.04	49.7		
All Vehic	cles	483	2.0	0.130	1.2	NA	0.7	4.9	0.18	0.09	47.2		

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

Processed: Tuesday, 3 March 2015 5:03:13 PM SIDRA INTERSECTION 5.1.13.2093

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Project: P:\13S1200-1299\13S1210200 - Bonds Pendle Hill Additional Modelling\Modelling\150302sid -

13S1210200 Jones Street-Smith Street Roundabout

Movem	ient Per	formance -	Vehicles								
Mov ID	Turn	Demand Flow	HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
South:	ones Str	ven/n	<u> </u>	V/C	sec		ven	m		per ven	Km/n
1	1	35	20	0.321	8.9	LOSA	19	13.7	0.62	0.75	41.5
2	T	186	2.0	0.321	7.9	LOSA	1.9	13.7	0.62	0.70	41.8
3	R	35	2.0	0.321	11.9	LOSA	1.9	13.7	0.62	0.84	39.6
Approac	h	256	2.0	0.321	8.6	LOS A	1.9	13.7	0.62	0.72	41.4
East: Sr	nith Stree	et									
4	L	69	2.0	0.391	7.4	LOS A	2.6	18.7	0.49	0.64	42.1
5	Т	249	2.0	0.391	6.4	LOS A	2.6	18.7	0.49	0.57	42.3
6	R	76	2.0	0.391	10.5	LOS A	2.6	18.7	0.49	0.76	40.5
Approad	h	395	2.0	0.391	7.4	LOS A	2.6	18.7	0.49	0.62	41.9
North: J	ones Stre	eet									
7	L	145	2.0	0.308	7.2	LOS A	1.9	13.6	0.44	0.61	42.2
8	Т	124	2.0	0.308	6.2	LOS A	1.9	13.6	0.44	0.54	42.5
9	R	41	2.0	0.308	10.2	LOS A	1.9	13.6	0.44	0.74	40.6
Approac	h	311	2.0	0.308	7.2	LOS A	1.9	13.6	0.44	0.60	42.1
West: S	mith Stre	et									
10	L	27	2.0	0.187	8.0	LOS A	1.0	7.3	0.52	0.68	42.1
11	Т	118	2.0	0.187	7.1	LOS A	1.0	7.3	0.52	0.62	42.2
12	R	14	2.0	0.187	11.1	LOS A	1.0	7.3	0.52	0.80	40.2
Approac	h	159	2.0	0.187	7.6	LOS A	1.0	7.3	0.52	0.64	42.0
All Vehic	cles	1120	2.0	0.391	7.6	LOS A	2.6	18.7	0.51	0.64	41.9

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.



13S1210200 Jones Street-Smith Street Roundabout

Movem	ent Per	formance - \	Vehicles								
	_	Demand		Deg.	Average	Level of	95% Back of	of Queue	Prop.	Effective	Average
Mov ID	lurn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
	01	veh/h	%	v/c	sec		veh	m		per veh	km/h
South: J	ones Str	eet			<i></i>						
1	L	24	2.0	0.198	7.1	LOS A	1.1	7.7	0.40	0.61	42.4
2	Т	128	2.0	0.198	6.2	LOS A	1.1	7.7	0.40	0.53	42.7
3	R	40	2.0	0.198	10.2	LOS A	1.1	7.7	0.40	0.76	40.7
Approac	:h	193	2.0	0.198	7.1	LOS A	1.1	7.7	0.40	0.59	42.2
East: Sn	nith Stree	et									
4	L	21	2.0	0.172	7.0	LOS A	0.9	6.6	0.39	0.60	42.4
5	Т	99	2.0	0.172	6.1	LOS A	0.9	6.6	0.39	0.52	42.8
6	R	47	2.0	0.172	10.1	LOS A	0.9	6.6	0.39	0.74	40.7
Approac	:h	167	2.0	0.172	7.3	LOS A	0.9	6.6	0.39	0.60	42.1
North: Jo	ones Stre	eet									
7	L	105	2.0	0.256	7.2	LOS A	1.5	10.5	0.42	0.62	42.3
8	Т	122	2.0	0.256	6.3	LOS A	1.5	10.5	0.42	0.54	42.6
9	R	23	2.0	0.256	10.3	LOS A	1.5	10.5	0.42	0.75	40.6
Approac	h	251	2.0	0.256	7.0	LOS A	1.5	10.5	0.42	0.60	42.3
West: Si	mith Stre	et									
10	L	13	2.0	0.160	7.4	LOS A	0.8	6.0	0.43	0.64	42.4
11	Т	118	2.0	0.160	6.4	LOS A	0.8	6.0	0.43	0.56	42.6
12	R	18	2.0	0.160	10.4	LOS A	0.8	6.0	0.43	0.78	40.6
Approac	:h	148	2.0	0.160	7.0	LOS A	0.8	6.0	0.43	0.59	42.4
All Vehic	cles	759	2.0	0.256	7.1	LOS A	1.5	10.5	0.41	0.59	42.2

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

Processed: Tuesday, 3 March 2015 5:03:13 PM SIDRA INTERSECTION 5.1.13.2093 Project: P:113S1200-1299\13S1210200 - Bonds Pendle Hill Additional Modelling\Modelling\150302sid - SIDRA ---

Project: P:\13S1200-1299\13S1210200 - Bonds Pendle Hill Additional Modelling\Modelling\150302sid -13S1210200 Bonds Spinning Mills - 2027 Base Conditions.sip 8000056, GTA CONSULTANTS, ENTERPRISE

13S1210200 Jones Street-Dunmore Street Roundabout

Moven	nent Per	formance	- Vehicles								
	_	Demand		Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
Mov ID	lurn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
0, 11		veh/h	%	v/c	sec	_	veh	m		per veh	km/h
South:	Jones Str	eet									
1	L	207	2.0	0.616	17.5	LOS B	5.6	40.1	0.95	1.11	35.2
2	Т	62	2.0	0.616	16.6	LOS B	5.6	40.1	0.95	1.11	35.3
3	R	41	2.0	0.616	20.6	LOS B	5.6	40.1	0.95	1.12	34.0
Approa	ch	311	2.0	0.616	17.8	LOS B	5.6	40.1	0.95	1.11	35.0
East: D	unmore S	Street									
4	L	73	2.0	0.796	14.1	LOS A	12.1	85.8	0.93	0.98	37.7
5	Т	627	2.0	0.796	13.2	LOS A	12.1	85.8	0.93	0.97	37.9
6	R	26	2.0	0.796	17.2	LOS B	12.1	85.8	0.93	1.00	36.4
Approa	ch	726	2.0	0.796	13.4	LOS A	12.1	85.8	0.93	0.97	37.8
North: J	ones Str	eet									
7	L	14	2.0	0.135	9.7	LOS A	0.7	5.2	0.65	0.74	40.4
8	Т	21	2.0	0.135	8.8	LOS A	0.7	5.2	0.65	0.70	40.7
9	R	56	2.0	0.135	12.8	LOS A	0.7	5.2	0.65	0.81	38.6
Approa	ch	91	2.0	0.135	11.4	LOS A	0.7	5.2	0.65	0.77	39.4
West: D	unmore	Street									
10	L	66	2.0	0.507	7.1	LOS A	4.3	30.7	0.51	0.60	42.0
11	Т	277	2.0	0.507	6.1	LOS A	4.3	30.7	0.51	0.53	42.1
12	R	218	2.0	0.507	10.1	LOS A	4.3	30.7	0.51	0.71	40.5
Approa	ch	561	2.0	0.507	7.8	LOS A	4.3	30.7	0.51	0.61	41.4
All Vehi	cles	1688	2.0	0.796	12.2	LOS A	12.1	85.8	0.78	0.87	38.4

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.



13S1210200 Jones Street-Dunmore Street Roundabout

Moven	nent Per	formance	- Vehicles								
		Demand		Deg.	Average	Level of	95% Back o	of Queue	Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
0, 11		veh/h	%	v/c	sec	_	veh	m	_	per veh	km/h
South:	Jones Str	eet									
1	L	135	2.0	0.258	9.4	LOS A	1.5	10.7	0.66	0.76	40.8
2	Т	23	2.0	0.258	8.5	LOS A	1.5	10.7	0.66	0.72	41.1
3	R	28	2.0	0.258	12.5	LOS A	1.5	10.7	0.66	0.83	39.0
Approa	ch	186	2.0	0.258	9.8	LOS A	1.5	10.7	0.66	0.77	40.5
East: D	unmore S	Street									
4	L	34	2.0	0.479	8.2	LOS A	3.3	23.7	0.59	0.71	42.0
5	Т	398	2.0	0.479	7.3	LOS A	3.3	23.7	0.59	0.65	42.0
6	R	18	2.0	0.479	11.3	LOS A	3.3	23.7	0.59	0.81	40.1
Approa	ch	449	2.0	0.479	7.5	LOS A	3.3	23.7	0.59	0.66	41.9
North: J	lones Stre	eet									
7	L	19	2.0	0.122	9.7	LOS A	0.6	4.5	0.62	0.73	40.5
8	Т	25	2.0	0.122	8.7	LOS A	0.6	4.5	0.62	0.69	40.9
9	R	40	2.0	0.122	12.7	LOS A	0.6	4.5	0.62	0.81	38.7
Approa	ch	84	2.0	0.122	10.9	LOS A	0.6	4.5	0.62	0.76	39.7
West: D	ounmore \$	Street									
10	L	40	2.0	0.437	6.4	LOS A	3.4	24.2	0.32	0.55	42.7
11	Т	324	2.0	0.437	5.5	LOS A	3.4	24.2	0.32	0.47	43.1
12	R	181	2.0	0.437	9.5	LOS A	3.4	24.2	0.32	0.71	40.9
Approa	ch	545	2.0	0.437	6.9	LOSA	3.4	24.2	0.32	0.55	42.3
All Vehi	cles	1265	2.0	0.479	7.8	LOS A	3.4	24.2	0.49	0.64	41.7

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.



Dunmore Street-Goodall Street

Signals - Fixed Time Cycle Time = 60 seconds (Optimum Cycle Time - Minimum Delay)

Movem	ent Pe	erformance -	Vehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
East: Du	Inmore	Street									
5	Т	535	2.0	0.647	12.9	LOS A	11.7	83.4	0.79	0.69	36.5
6	R	311	2.0	0.800	31.9	LOS C	9.0	64.3	1.00	1.02	28.3
Approac	h	845	2.0	0.800	19.9	LOS B	11.7	83.4	0.87	0.81	33.0
North: G	oodall	Street									
7	L	291	2.0	0.507	14.9	LOS B	4.7	33.8	0.59	0.75	36.8
9	R	476	2.0	0.780	29.5	LOS C	14.1	100.6	0.96	0.93	29.2
Approac	h	766	2.0	0.780	24.0	LOS B	14.1	100.6	0.82	0.86	31.7
West: D	unmore	Street									
10	L	277	2.0	0.257	9.9	LOS A	2.9	20.3	0.38	0.70	40.4
11	Т	277	2.0	0.539	21.0	LOS B	7.2	51.1	0.91	0.76	31.8
Approac	h	554	2.0	0.539	15.4	LOS B	7.2	51.1	0.64	0.73	35.6
All Vehic	les	2165	2.0	0.800	20.2	LOS B	14.1	100.6	0.79	0.81	33.1

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	nent Performance -	Pedestrians	;					
May ID	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective
	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P5	Across N approach	53	24.3	LOS C	0.1	0.1	0.90	0.90
P7	Across W approach	53	20.8	LOS C	0.1	0.1	0.83	0.83
All Ped	estrians	106	22.6	LOS C			0.87	0.87

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.



Dunmore Street-Goodall Street

Signals - Fixed Time Cycle Time = 56 seconds (Optimum Cycle Time - Minimum Delay)

Movem	ent Pe	erformance - V	/ehicles								
Mov ID	Turn	Demand Flow veh/ <u>h</u>	HV %	Deg. Satn v/c	Average Delay se <u>c</u>	Level of Service	95% Back c Vehicles veh	f Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/ <u>h</u>
East: Du	inmore	Street									
5	Т	336	2.0	0.377	9.2	LOS A	5.6	40.0	0.64	0.55	39.4
6	R	240	2.0	0.583	22.7	LOS B	5.5	39.1	0.93	0.82	32.3
Approac	h	576	2.0	0.583	14.8	LOS B	5.6	40.0	0.77	0.66	36.1
North: G	oodall	Street									
7	L	233	2.0	0.407	15.2	LOS B	3.7	26.2	0.61	0.74	36.6
9	R	251	2.0	0.479	24.9	LOS B	5.9	41.8	0.88	0.81	31.3
Approac	h	483	2.0	0.479	20.2	LOS B	5.9	41.8	0.75	0.78	33.6
West: Di	unmore	Street									
10	L	198	2.0	0.192	9.9	LOS A	1.9	13.8	0.38	0.69	40.4
11	Т	322	2.0	0.586	19.1	LOS B	7.8	55.7	0.91	0.77	32.7
Approac	h	520	2.0	0.586	15.6	LOS B	7.8	55.7	0.71	0.74	35.3
All Vehic	les	1579	2.0	0.586	16.7	LOS B	7.8	55.7	0.74	0.72	35.0

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians												
MoviD	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective				
	Description	FIOW	Delay	Service	Pedestrian	Distance	Queued	Stop Rate				
		ped/h	sec		ped	m		per ped				
P5	Across N approach	53	22.3	LOS C	0.1	0.1	0.89	0.89				
P7	Across W approach	53	22.3	LOS C	0.1	0.1	0.89	0.89				
All Ped	estrians	106	22.3	LOS C			0.89	0.89				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.



Dunmore Street-Pendle Way

Signals - Fixed Time Cycle Time = 60 seconds (Optimum Cycle Time - Minimum Delay)

Movement Performance - Vehicles											
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: P	endle \	Nay									
2	Т	331	2.0	0.368	11.2	LOS A	6.3	44.8	0.69	0.59	37.9
3	R	304	2.0	0.725	26.6	LOS B	8.3	59.0	0.98	0.91	30.5
Approac	h	635	2.0	0.725	18.6	LOS B	8.3	59.0	0.83	0.74	33.9
East: Du	inmore	Street									
4	L	502	2.0	0.514	16.2	LOS B	9.5	68.0	0.69	0.80	36.0
6	R	476	2.0	0.780	29.5	LOS C	14.1	100.6	0.96	0.93	29.2
Approac	h	978	2.0	0.780	22.7	LOS B	14.1	100.6	0.82	0.86	32.3
North: P	endle V	Vay									
7	L	238	2.0	0.221	9.8	LOS A	2.4	17.0	0.37	0.70	40.5
8	Т	224	2.0	0.437	20.3	LOS B	5.6	40.0	0.88	0.72	32.2
Approac	h	462	2.0	0.437	14.9	LOS B	5.6	40.0	0.61	0.71	36.0
All Vehic	les	2075	2.0	0.780	19.7	LOS B	14.1	100.6	0.78	0.79	33.6

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians													
	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective					
	Description	FIOW	Delay	Service	Pedestrian	Distance	Queuea	Stop Rate					
		ped/h	sec		ped	m		per ped					
P1	Across S approach	53	12.0	LOS B	0.1	0.1	0.63	0.63					
P3	Across E approach	53	24.3	LOS C	0.1	0.1	0.90	0.90					
All Pede	estrians	106	18.2	LOS B			0.77	0.77					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.



Dunmore Street-Pendle Way

Signals - Fixed Time Cycle Time = 50 seconds (Optimum Cycle Time - Minimum Delay)

Movement Performance - Vehicles											
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: F	Pendle V	Vay	,,,								
2	Т	362	2.0	0.556	6.7	LOS A	5.3	37.9	0.59	0.50	41.3
3	R	259	2.0	0.556	17.4	LOS B	5.3	37.9	0.86	0.82	35.3
Approac	h	621	2.0	0.556	11.2	LOS A	5.3	37.9	0.70	0.63	38.6
East: Du	inmore	Street									
4	L	304	2.0	0.378	16.8	LOS B	5.1	36.3	0.71	0.78	35.6
6	R	284	2.0	0.776	31.0	LOS C	7.6	54.0	1.00	0.95	28.6
Approac	h	588	2.0	0.776	23.6	LOS B	7.6	54.0	0.85	0.86	31.9
North: P	endle W	Vay									
7	L	271	2.0	0.252	10.6	LOS A	2.8	20.0	0.45	0.72	39.9
8	Т	265	2.0	0.431	14.9	LOS B	5.3	37.5	0.83	0.70	35.2
Approac	h	536	2.0	0.431	12.7	LOS A	5.3	37.5	0.64	0.71	37.4
All Vehic	les	1745	2.0	0.776	15.9	LOS B	7.6	54.0	0.73	0.73	35.7

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian	of Queue Distance m	Prop. Queued	Effective Stop Rate					
P1	Across S approach	53	14.4	LOS B	0.1	0.1	0.76	0.76					
All Ped	Across E approach estrians	106	19.4 16.9	LOS B	0.1	0.1	0.88	0.88					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.



13S1210200 - Bonds Pendle Hill Additional Modelling Great Western Highway / Jones Street 2027 Base Condition Thursday PM Peak Signals - Fixed Time Cycle Time = 140 seconds (User-Given Phase Times)

Movem	ent Per	formance - V	/ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: E	Berith Rd	- S									
4	L	51	2.1	0.499	55.4	LOS D	2.8	19.8	0.84	0.73	24.2
5	Т	21	0.0	0.399	61.2	LOS E	5.2	36.8	0.96	0.75	19.2
6	R	60	1.8	0.399	68.9	LOS E	5.2	36.8	0.96	0.78	21.5
Approac	h	132	1.6	0.499	62.5	LOS E	5.2	36.8	0.91	0.76	22.1
East: Gt	Westerr	ı Hwy - E									
7	L	71	0.0	0.574	23.0	LOS B	26.3	186.1	0.58	1.09	45.4
8	Т	2153	1.6	0.574	12.9	LOS A	26.5	188.1	0.58	0.53	53.1
9	R	136	0.0	0.539	49.1	LOS D	6.7	46.7	0.98	0.79	27.6
Approac	h	2359	1.5	0.574	15.3	LOS B	26.5	188.1	0.60	0.57	50.6
North: J	ones St -	N									
10	L	71	0.0	0.360	38.8	LOS C	3.1	21.9	0.69	0.73	29.0
11	Т	35	0.0	0.767	72.0	LOS F	8.9	62.4	1.00	0.90	17.4
12	R	88	1.2	0.767	79.8	LOS F	8.9	62.4	1.00	0.90	19.6
Approac	h	194	0.5	0.767	63.4	LOS E	8.9	62.4	0.89	0.84	21.8
West: G	t Wester	n Hwy - W									
1	L	53	0.0	0.467	30.2	LOS C	19.8	140.5	0.65	1.05	39.6
2	Т	1423	2.0	0.467	20.1	LOS B	20.0	142.4	0.65	0.58	45.7
3	R	51	2.1	0.483	83.0	LOS F	3.5	25.2	1.00	0.75	19.0
Approac	:h	1526	1.9	0.483	22.5	LOS B	20.0	142.4	0.66	0.60	43.8
All Vehic	cles	4211	1.6	0.767	21.6	LOS B	26.5	188.1	0.65	0.60	43.8

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped					
P3	Across S approach	53	11.2	LOS B	0.1	0.1	0.40	0.40					
P7	Across N approach	53	19.0	LOS B	0.1	0.1	0.52	0.52					
P1	Across W approach	53	60.4	LOS F	0.2	0.2	0.93	0.93					
All Ped	estrians	159	30.2	LOS D			0.62	0.62					



13S1210200 - Bonds Pendle Hill Additional Modelling Great Western Highway / Jones Street 2027 Base Condition Saturday Peak Signals - Fixed Time Cycle Time = 130 seconds (User-Given Phase Times)

Movem	ent Per	formance - \	/ ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay se <u>c</u>	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/ <u>h</u>
South: E	Berith Rd	- S									
4	L	75	1.4	0.694	56.3	LOS D	4.2	29.5	0.87	0.83	21.2
5	Т	18	0.0	0.569	63.0	LOS E	5.5	39.6	1.00	0.79	21.1
6	R	69	3.0	0.569	69.1	LOS E	5.5	39.6	1.00	0.79	18.9
Approac	h	162	1.9	0.694	62.5	LOS E	5.5	39.6	0.94	0.80	20.2
East: Gt	Western	n Hwy - E									
7	L	88	1.2	0.317	24.2	LOS B	14.7	105.5	0.65	1.04	44.7
8	Т	1119	3.1	0.317	17.2	LOS B	16.2	116.3	0.68	0.67	47.8
9	R	97	3.3	0.408	49.3	LOS D	4.7	34.0	0.96	0.77	27.5
Approac	h	1304	3.0	0.408	20.0	LOS B	16.2	116.3	0.70	0.70	45.2
North: J	ones St -	N									
10	L	78	0.0	0.392	40.0	LOS C	3.4	23.8	0.73	0.74	28.6
11	Т	35	0.0	0.751	71.4	LOS F	5.3	37.7	1.00	0.87	17.6
12	R	42	5.0	0.751	79.3	LOS F	5.3	37.7	1.00	0.87	19.8
Approac	h	155	1.4	0.751	57.7	LOS E	5.3	37.7	0.87	0.81	22.9
West: G	t Westeri	n Hwy - W									
1	L	33	0.0	0.428	26.4	LOS B	16.5	117.1	0.60	1.07	42.4
2	Т	1392	1.8	0.428	16.3	LOS B	16.6	118.2	0.60	0.54	49.4
3	R	72	1.5	0.460	73.7	LOS F	4.5	32.0	0.99	0.77	20.8
Approac	:h	1496	1.8	0.460	19.3	LOS B	16.6	118.2	0.62	0.56	46.7
All Vehic	cles	3117	2.3	0.751	23.7	LOS B	16.6	118.2	0.68	0.65	41.4

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	nent Performance -	Pedestrians	i					
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped
P3	Across S approach	53	10.8	LOS B	0.1	0.1	0.41	0.41
P7	Across N approach	53	16.3	LOS B	0.1	0.1	0.50	0.50
P1	Across W approach	53	58.2	LOS E	0.2	0.2	0.95	0.95
All Ped	estrians	159	28.4	LOS C			0.62	0.62



13S1210200 - Bonds Pendle Hill Additional Modelling Cumberland Highway / Dunmore Street 2027 Base Condition Thursday PM Peak Signals - Fixed Time Cycle Time = 140 seconds (User-Given Phase Times)

Movem	Movement Performance - Vehicles											
MarialD	т	Demand	1117	Deg.	Average	Level of	95% Back (of Queue	Prop.	Effective	Average	
	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed	
O suther C)	veh/h	%	V/C	sec		veh	m		per veh	km/h	
South: C	umberia	and Hwy - S		. =		100 5	10 -					
1	L	58	0.0	0.780	45.4	LOS D	40.5	289.3	0.88	0.96	29.5	
2	Т	2180	2.6	0.780	37.6	LOS C	40.8	292.2	0.88	0.80	30.6	
3	R	49	0.0	0.287	74.1	LOS F	3.2	22.7	0.97	0.75	19.9	
Approac	h	2287	2.5	0.780	38.6	LOS C	40.8	292.2	0.88	0.80	30.2	
East: Du	inmore \$	St - E										
4	L	74	0.0	0.757	41.4	LOS C	17.2	121.4	0.76	0.87	27.4	
5	Т	528	1.0	1.009	58.7	LOS E	44.8	315.1	0.87	0.92	19.9	
6	R	302	0.3	1.009	93.3	LOS F	44.8	315.1	1.00	1.20	17.0	
Approac	h	904	0.7	1.009	68.9	LOS E	44.8	315.1	0.91	1.01	19.2	
North: C	umberla	and Hwy - N										
7	L	347	1.2	0.579	25.4	LOS B	8.9	62.8	0.49	0.77	37.3	
8	Т	2403	3.9	0.840	39.5	LOS C	46.6	337.0	0.92	0.85	29.8	
9	R	168	3.1	0.696	40.9	LOS C	6.6	47.2	1.00	0.85	29.2	
Approac	h	2919	3.5	0.840	37.9	LOS C	46.6	337.0	0.88	0.84	30.5	
West: D	unmore	St - W										
10	L	104	0.0	0.639	62.8	LOS E	13.7	96.4	0.96	0.83	21.8	
11	Т	267	1.6	0.639	55.7	LOS D	13.7	96.4	0.97	0.81	20.5	
12	R	48	4.3	0.639	63.6	LOS E	12.8	91.3	0.97	0.83	21.8	
Approac	h	420	1.5	0.639	58.4	LOS E	13.7	96.4	0.97	0.82	21.0	
All Vehic	les	6531	2.6	1.009	43.7	LOS D	46.6	337.0	0.89	0.85	27.4	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Mover	Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped					
P1	Across S approach	53	39.4	LOS D	0.2	0.2	0.75	0.75					
P3	Across E approach	53	32.9	LOS D	0.1	0.1	0.69	0.69					
P5	Across N approach	53	58.5	LOS E	0.2	0.2	0.91	0.91					
P7	Across W approach	53	32.9	LOS D	0.1	0.1	0.69	0.69					
All Ped	estrians	212	40.9	LOS E			0.76	0.76					



13S1210200 - Bonds Pendle Hill Additional Modelling Cumberland Highway / Dunmore Street 2027 Base Condition Saturday Peak Signals - Fixed Time Cycle Time = 130 seconds (User-Given Phase Times)

Movem	ent Pe	rformance - '	Vehicles								
Mov ID	Turn	Demand Flow	HV	Deg. Satn	Average Delay	Level of Service	95% Back (Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
Cauthy C		veh/h	%	V/C	sec		veh	m		per veh	km/h
South: C	Jumpena	and Hwy - S	0.0	0 705	40.0		00.0	011.0	0.00	0.00	00.0
1	L _	27	0.0	0.795	46.3	LOS D	30.0	214.3	0.90	0.96	29.2
2	I	1655	2.7	0.795	38.5	LOS C	30.1	215.8	0.90	0.83	30.2
3	R	81	1.3	0.636	76.1	LOS F	5.3	37.7	1.00	0.80	19.5
Approac	h	1763	2.6	0.795	40.4	LOS C	30.1	215.8	0.90	0.83	29.5
East: Du	Inmore S	St - E									
4	L	63	3.3	0.641	34.2	LOS C	8.3	58.5	0.70	0.82	30.0
5	Т	289	0.4	1.068	79.3	LOS F	43.3	304.6	0.85	0.99	16.6
6	R	313	0.7	1.068	135.4	LOS F	43.3	304.6	1.00	1.38	13.0
Approac	:h	665	0.8	1.068	101.4	LOS F	43.3	304.6	0.91	1.16	15.2
North: C	umberla	and Hwy - N									
7	L	353	1.5	0.752	32.6	LOS C	13.6	96.5	0.70	0.82	33.0
8	Т	1725	2.4	0.811	39.4	LOS C	31.5	225.3	0.91	0.84	29.9
9	R	131	0.0	0.651	39.9	LOS C	5.0	34.8	0.98	0.82	29.6
Approac	:h	2208	2.1	0.811	38.4	LOS C	31.5	225.3	0.88	0.84	30.3
West: D	unmore	St - W									
10	L	111	0.0	0.620	57.2	LOS E	13.2	92.4	0.95	0.83	23.0
11	Т	243	0.4	0.620	49.7	LOS D	13.2	92.4	0.95	0.80	21.7
12	R	84	1.3	0.620	57.2	LOS E	12.1	85.1	0.96	0.83	23.1
Approac	:h	438	0.5	0.620	53.0	LOS D	13.2	92.4	0.95	0.81	22.3
All Vehic	cles	5075	1.9	1.068	48.6	LOS D	43.3	304.6	0.90	0.87	25.8

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped					
P1	Across S approach	53	34.7	LOS D	0.1	0.1	0.73	0.73					
P3	Across E approach	53	32.6	LOS D	0.1	0.1	0.71	0.71					
P5	Across N approach	53	53.6	LOS E	0.2	0.2	0.91	0.91					
P7	Across W approach	53	32.6	LOS D	0.1	0.1	0.71	0.71					
All Pedestrians		212	38.3	LOS D			0.76	0.76					



13S1210200 - Bonds Pendle Hill Additional Modelling Cumberland Highway / Smith Street 2027 Base Condition Thursday PM Peak Signals - Fixed Time Cycle Time = 140 seconds (User-Given Phase Times)

Movement Performance - Vehicles											
Mov ID	Turn	Demand Flow	HV %	Deg. Satn	Average Delay	Level of Service	95% Back (Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed km/b
South: 0	Cumberla	nd Hwy - S	/0	V/C	300		VCII			perven	IX11//11
1	L	131	0.8	0.667	18.5	LOS B	21.8	155.7	0.45	0.96	41.0
2	Т	2267	2.6	0.667	9.2	LOS A	21.8	155.7	0.41	0.38	46.3
3	R	64	0.0	0.440	77.0	LOS F	4.4	30.5	0.99	0.76	19.2
Approac	ch	2462	2.5	0.667	11.5	LOS A	21.8	155.7	0.43	0.42	44.4
East: Sr	nith St - I	=									
4	L	67	0.0	0.527	64.0	LOS E	6.9	48.5	0.93	0.79	22.0
5	Т	280	1.5	1.055	131.8	LOS F	29.0	205.4	0.99	1.24	12.7
6	R	34	0.0	1.055	154.5	LOS F	29.0	205.4	1.00	1.34	11.7
Approad	ch	381	1.1	1.055	121.8	LOS F	29.0	205.4	0.98	1.17	13.6
North: C	Cumberla	nd Hwy - N									
7	L	49	2.1	0.660	17.5	LOS B	19.9	144.0	0.41	1.00	41.8
8	Т	2313	3.9	0.660	8.9	LOS A	19.9	144.0	0.40	0.37	46.7
9	R	56	1.9	0.387	76.7	LOS F	3.8	26.8	0.99	0.75	19.3
Approac	ch	2418	3.8	0.660	10.6	LOS A	19.9	144.0	0.41	0.39	45.1
West: S	mith St -	W									
10	L	36	0.0	0.461	63.6	LOS E	5.9	42.1	0.92	0.80	22.3
11	Т	131	4.0	0.923	72.6	LOS F	8.9	63.5	0.96	0.88	19.1
12	R	41	0.0	0.923	95.8	LOS F	8.9	63.5	1.00	1.02	16.9
Approac	ch	207	2.5	0.923	75.6	LOS F	8.9	63.5	0.96	0.89	19.1
All Vehic	cles	5468	3.0	1.055	21.2	LOS B	29.0	205.4	0.48	0.48	36.9

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped					
P1	Across S approach	53	59.4	LOS E	0.2	0.2	0.92	0.92					
P3	Across E approach	53	13.7	LOS B	0.1	0.1	0.44	0.44					
P5	Across N approach	53	59.4	LOS E	0.2	0.2	0.92	0.92					
P7	Across W approach	53	13.7	LOS B	0.1	0.1	0.44	0.44					
All Pedestrians		212	36.6	LOS D			0.68	0.68					



13S1210200 - Bonds Pendle Hill Additional Modelling Cumberland Highway / Smith Street 2027 Base Condition Saturday Peak Signals - Fixed Time Cycle Time = 130 seconds (User-Given Phase Times)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h	
South: C	Cumberla	and Hwy - S										
1	L	133	0.8	0.518	20.1	LOS B	20.9	148.7	0.56	0.93	39.9	
2	Т	1836	2.5	0.518	11.9	LOS A	21.0	150.3	0.56	0.51	43.3	
3	R	64	3.3	0.460	72.9	LOS F	4.1	29.4	1.00	0.76	20.0	
Approac	h	2033	2.4	0.518	14.4	LOS A	21.0	150.3	0.57	0.54	41.5	
East: Sr	nith St -	E										
4	L	46	2.3	0.189	49.7	LOS D	2.3	16.5	0.82	0.74	25.4	
5	Т	101	0.0	0.663	63.0	LOS E	7.9	55.4	1.00	0.83	21.0	
6	R	23	0.0	0.663	71.0	LOS F	7.9	55.4	1.00	0.83	21.0	
Approac	h	171	0.6	0.663	60.4	LOS E	7.9	55.4	0.95	0.80	22.1	
North: C	umberla	and Hwy - N										
7	L	48	0.0	0.482	19.7	LOS B	18.8	134.3	0.54	0.97	40.4	
8	Т	1784	2.6	0.482	11.5	LOS A	18.8	134.9	0.54	0.49	43.8	
9	R	64	0.0	0.449	72.7	LOS F	4.1	28.5	1.00	0.76	20.0	
Approac	:h	1897	2.4	0.482	13.8	LOS A	18.8	134.9	0.55	0.51	42.0	
West: S	mith St -	W										
10	L	28	0.0	0.227	61.3	LOS E	2.9	20.1	0.92	0.76	22.7	
11	Т	95	0.0	0.796	65.9	LOS E	7.8	55.1	0.98	0.85	20.3	
12	R	42	2.5	0.796	77.9	LOS F	7.8	55.1	1.00	0.90	19.6	
Approac	:h	165	0.6	0.796	68.2	LOS E	7.8	55.1	0.97	0.85	20.5	
All Vehic	cles	4265	2.3	0.796	18.0	LOS B	21.0	150.3	0.59	0.55	38.8	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped					
P1	Across S approach	53	59.1	LOS E	0.2	0.2	0.95	0.95					
P3	Across E approach	53	11.2	LOS B	0.1	0.1	0.42	0.42					
P5	Across N approach	53	59.1	LOS E	0.2	0.2	0.95	0.95					
P7	Across W approach	53	11.2	LOS B	0.1	0.1	0.42	0.42					
All Pedestrians		212	35.2	LOS D			0.68	0.68					



13S1210200 - Bonds Pendle Hill Additional Modelling Gilba Road / Pendle Way 2027 Base Condition Thursday PM Peak Giveway / Yield (Two-Way)

Movement Performance - Vehicles													
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h		
South: F	endle V	Vay - S											
1	L	512	0.8	0.277	8.2	LOS A	0.0	0.0	0.00	0.67	49.0		
2	Т	226	0.5	0.407	13.5	LOS A	2.3	16.3	0.63	0.92	43.7		
Approac	h	738	0.7	0.407	9.8	LOS A	2.3	16.3	0.19	0.75	47.2		
North: P	endle W	/ay - N											
8	Т	173	3.7	0.591	22.4	LOS B	4.2	30.2	0.70	1.08	37.0		
9	R	60	1.8	0.591	23.7	LOS B	4.2	30.2	0.70	1.08	36.6		
Approac	h	233	3.2	0.591	22.7	LOS B	4.2	30.2	0.70	1.08	36.9		
West: G	ilba Rd	- W											
10	L	193	0.5	0.252	8.2	LOS A	0.0	0.0	0.00	0.66	49.0		
12	R	272	1.2	0.252	8.3	LOS A	0.0	0.0	0.00	0.67	48.9		
Approac	h	464	0.9	0.252	8.2	NA	0.0	0.0	0.00	0.67	48.9		
All Vehic	les	1435	1.2	0.591	11.4	NA	4.2	30.2	0.21	0.78	45.7		

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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13S1210200 - Bonds Pendle Hill Additional Modelling Gilba Road / Pendle Way 2027 Base Condition Saturday Peak Giveway / Yield (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h	
South: F	Pendle V	Vay - S										
1	L	332	1.9	0.181	8.3	LOS A	0.0	0.0	0.00	0.67	49.0	
2	Т	231	0.0	0.482	15.8	LOS B	2.7	18.9	0.69	0.98	41.7	
Approac	h	562	1.1	0.482	11.4	LOS A	2.7	18.9	0.28	0.80	45.7	
North: P	endle V	Vay - N										
8	Т	172	3.1	0.686	25.7	LOS B	5.2	37.3	0.78	1.22	35.0	
9	R	85	1.2	0.686	27.0	LOS B	5.2	37.3	0.78	1.18	34.6	
Approac	h	257	2.5	0.686	26.1	LOS B	5.2	37.3	0.78	1.20	34.9	
West: G	ilba Rd	- W										
10	L	201	1.0	0.274	8.2	LOS A	0.0	0.0	0.00	0.66	49.0	
12	R	306	0.3	0.274	8.2	LOS A	0.0	0.0	0.00	0.67	48.9	
Approac	h	507	0.6	0.274	8.2	NA	0.0	0.0	0.00	0.67	48.9	
All Vehic	les	1326	1.2	0.686	13.0	NA	5.2	37.3	0.27	0.83	44.2	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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13S1210200 - Bonds Pendle Hill Additional Modelling Goodall Street / Wentworth Avenue 2027 Base Condition Thursday PM Peak Signals - Fixed Time Cycle Time = 90 seconds (User-Given Phase Times)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h	
South: G	Goodall S	St - S										
1	L	360	1.5	0.610	15.7	LOS B	7.1	50.5	0.52	0.75	37.4	
3	R	291	0.7	0.832	51.9	LOS D	13.9	97.9	1.00	0.95	23.1	
Approac	h	651	1.1	0.832	31.9	LOS C	13.9	97.9	0.73	0.84	29.3	
East: We	entworth	n Ave - E										
4	L	367	0.3	0.866	33.9	LOS C	11.6	81.6	0.84	0.87	30.0	
5	Т	382	2.5	0.711	35.2	LOS C	15.8	113.2	0.98	0.85	29.1	
Approac	h	749	1.4	0.866	34.5	LOS C	15.8	113.2	0.91	0.86	29.5	
West: W	entwort	h Ave - W										
11	Т	395	2.4	0.540	6.4	LOS A	6.6	47.1	0.45	0.39	49.1	
12	R	539	2.0	0.649	32.4	LOS C	19.5	138.9	0.88	0.85	30.7	
Approac	h	934	2.1	0.649	21.4	LOS B	19.5	138.9	0.70	0.66	36.7	
All Vehic	les	2334	1.6	0.866	28.6	LOS C	19.5	138.9	0.78	0.77	31.9	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians												
		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective				
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate				
		ped/h	sec		ped	m		per ped				
P7	Across W approach	53	36.5	LOS D	0.1	0.1	0.90	0.90				
All Ped	estrians	53	36.5	LOS D			0.90	0.90				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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13S1210200 - Bonds Pendle Hill Additional Modelling Goodall Street / Wentworth Avenue 2027 Base Condition Saturday Peak Signals - Fixed Time Cycle Time = 90 seconds (User-Given Phase Times)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h	
South: C	Goodall	St - S										
1	L	357	1.5	0.605	16.6	LOS B	7.0	49.9	0.51	0.77	41.2	
3	R	263	0.8	0.916	64.2	LOS E	14.2	100.2	1.00	1.04	21.7	
Approac	h	620	1.2	0.916	36.8	LOS C	14.2	100.2	0.72	0.88	29.8	
East: We	entworth	h Ave - E										
4	L	236	0.4	0.576	23.9	LOS B	6.2	43.3	0.64	0.77	36.2	
5	Т	358	2.6	0.615	33.6	LOS C	14.2	101.7	0.95	0.80	29.8	
Approac	h	594	1.8	0.615	29.7	LOS C	14.2	101.7	0.83	0.79	32.0	
West: W	/entwort	h Ave - W										
11	Т	379	2.5	0.468	4.8	LOS A	5.5	39.5	0.38	0.32	51.3	
12	R	371	2.8	0.381	28.1	LOS B	11.3	80.8	0.74	0.81	33.9	
Approac	h	749	2.7	0.468	16.3	LOS B	11.3	80.8	0.56	0.56	40.9	
All Vehic	les	1963	1.9	0.916	26.9	LOS B	14.2	101.7	0.69	0.73	34.0	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians												
	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective				
	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate				
		ped/h	sec		ped	m		per ped				
P7	Across W approach	53	39.2	LOS D	0.1	0.1	0.93	0.93				
All Ped	estrians	53	39.2	LOS D			0.93	0.93				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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13S1210200 - Bonds Pendle Hill Additional Modelling Great Western Highway / Pendle Way 2027 Base Condition Thursday PM Peak Signals - Fixed Time Cycle Time = 140 seconds (User-Given Phase Times)

Movem	ient Per	formance -	Vehicles								
Mov ID	Turn	Demand Flow	HV	Deg. Satn	Average Delay	Level of Service	95% Back of Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
South: F	-ttalong F	Rd - S	70	V/C	Sec	_	ven	111	_	per ven	K111/11
4	L	124	0.8	0.606	53.5	LOS D	6.7	47.5	0.84	0.77	24.2
5	т	244	0.9	1.099	111.6	LOS F	25.1	176.2	1.00	1.05	14.3
6	R	123	0.0	1.099	187.0	LOS F	25.1	176.2	1.00	1.36	9.9
Approac	h	492	0.6	1.099	115.8	LOS F	25.1	176.2	0.96	1.06	14.2
East: Gt	Westerr	n Hwy - E									
7	L	261	0.4	0.695	41.9	LOS C	32.3	228.9	0.82	0.95	28.6
8	Т	1748	2.0	0.695	31.3	LOS C	32.7	232.6	0.79	0.72	30.8
<mark>9</mark>	R	<mark>156</mark>	1.3	1.000 ³	73.0	LOS F	10.4	73.4	1.00	0.80	20.0
Approac	h	2165	1.8	1.000	35.6	LOS C	32.7	232.6	0.81	0.75	29.4
North: P	endle Wa	ay - N									
10	L	91	1.2	0.741	59.9	LOS E	17.5	123.7	0.96	0.88	23.3
11	Т	316	1.3	0.926	60.0	LOS E	34.7	248.5	0.98	0.91	21.4
12	R	328	3.2	0.926	82.9	LOS F	34.7	248.5	1.00	1.01	18.5
Approac	h	735	2.1	0.926	70.2	LOS E	34.7	248.5	0.99	0.95	20.2
West: G	t Wester	n Hwy - W									
1	L	133	6.3	0.539	40.6	LOS C	21.2	152.8	0.80	0.89	29.3
2	Т	1159	2.4	0.539	32.2	LOS C	22.0	157.0	0.80	0.71	30.4
3	R	134	0.8	0.633	73.8	LOS F	9.0	63.4	1.00	0.81	19.9
Approac	h	1425	2.6	0.633	36.9	LOS C	22.0	157.0	0.82	0.73	28.9
All Vehic	cles	4817	1.9	1.099	49.4	LOS D	34.7	248.5	0.85	0.81	24.8

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

3 x = 1.00 due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

Movement Performance - Pedestrians											
		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective			
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate			
		ped/h	sec		ped	m		per ped			
P3	Across S approach	53	31.6	LOS D	0.1	0.1	0.67	0.67			
P5	Across E approach	53	59.4	LOS E	0.2	0.2	0.92	0.92			
P7	Across N approach	53	29.6	LOS C	0.1	0.1	0.65	0.65			
All Pedestrians 159			40.2	LOS E			0.75	0.75			



13S1210200 - Bonds Pendle Hill Additional Modelling Great Western Highway / Pendle Way 2027 Base Condition Saturday Peak Signals - Fixed Time Cycle Time = 130 seconds (User-Given Phase Times)

Movement Performance - Vehicles											
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/ <u>h</u>
South: E	Ettalong I	Rd - S									
4	L	77	1.4	0.328	42.1	LOS C	3.4	24.4	0.75	0.75	27.8
5	Т	195	0.5	0.952	67.7	LOS E	18.4	129.8	0.98	0.89	20.1
6	R	163	1.3	0.952	93.5	LOS F	18.4	129.8	1.00	1.11	17.0
Approac	h	435	1.0	0.952	72.8	LOS F	18.4	129.8	0.95	0.95	19.7
East: Gt	Westerr	n Hwy - E									
7	L	171	0.6	0.460	38.1	LOS C	15.0	106.5	0.81	0.90	29.9
8	Т	908	3.5	0.460	31.9	LOS C	17.2	123.7	0.81	0.71	30.5
<mark>9</mark>	R	<mark>168</mark>	2.7	<mark>1.000</mark> ໍ	66.0	LOS E	10.3	73.4	0.99	0.80	21.3
Approac	:h	1247	3.0	1.000	37.4	LOS C	17.2	123.7	0.83	0.75	28.7
North: P	endle W	'ay - N									
10	L	137	3.1	0.660	43.1	LOS D	9.6	68.7	0.98	0.82	28.0
11	Т	194	0.5	0.825	50.6	LOS D	19.0	134.3	0.99	0.88	23.4
12	R	174	1.8	0.825	69.7	LOS E	19.0	134.3	1.00	0.93	20.9
Approac	h	504	1.7	0.825	55.2	LOS D	19.0	134.3	0.99	0.88	23.5
West: G	t Wester	n Hwy - W									
1	L	129	1.6	0.595	43.0	LOS D	21.2	150.5	0.86	0.88	28.4
2	Т	1154	1.9	0.595	34.7	LOS C	21.8	155.4	0.86	0.76	29.3
3	R	105	1.0	0.481	64.2	LOS E	6.2	44.0	0.96	0.78	21.7
Approac	h	1388	1.8	0.595	37.7	LOS C	21.8	155.4	0.87	0.77	28.5
All Vehic	cles	3575	2.1	1.000	44.3	LOS D	21.8	155.4	0.88	0.80	26.3

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

3 x = 1.00 due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

Movement Performance - Pedestrians											
		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective			
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate			
		ped/h	sec		ped	m		per ped			
P3	Across S approach	53	34.0	LOS D	0.1	0.1	0.72	0.72			
P5	Across E approach	53	59.1	LOS E	0.2	0.2	0.95	0.95			
P7	Across N approach	53	31.9	LOS D	0.1	0.1	0.70	0.70			
All Pedestrians 159			41.7	LOS E			0.79	0.79			





Attachment 3C – SIDRA Output (2027 Post Development Thursday PM & Saturday Midday)

Intersection	Peak	Leg	Degree of Saturation (DOS)	Average Delay (sec)	95 th Percentile (m)	Level of Service (LOS)
		South	0.20	9	10	А
	DNA	East	0.04	9	1	А
	PIVI	North	0.20	7	0	А
Jones Street/		All	0.20	1	10	А
Rogers Street		South	0.15	8	7	А
	Cat	East	0.04	8	1	А
	Sat	North	0.17	7	0	А
		All	0.17	2	7	А
		South	0.28	10	18	А
	DM	East	0.09	11	2	А
	PIVI	North	0.25	7	0	А
Jones Street/		All	0.27	2	17	А
Oatlands Street	Sat	South	0.25	10	16	А
		East	0.08	12	2	А
		North	0.28	7	0	А
		All	0.28	2	16	А
	PM	South	0.53	15	30	В
		East	0.52	12	27	А
		North	0.42	10	21	А
		West	0.29	13	12	А
Jones Street/		All	0.53	9	30	А
Smith Street		South	0.36	12	16	А
		East	0.31	12	13	А
	Sat	North	0.49	11	27	А
		West	0.27	12	11	А
		All	0.49	8	27	А
		South	0.79	31	68	С
		East	0.92	27	165	В
	PM	North	0.15	14	6	А
		West	0.57	10	37	А
Jones Street/		All	0.92	19	165	В
Dunmore Street		South	0.39	14	18	А
		East	0.61	13	40	А
	Sat	North	0.14	14	6	A
		West	0.56	10	37	A
		All	0.61	9	40	А



Intersection	Peak	Leg	Degree of Saturation (DOS)	Average Delay (sec)	95 th Percentile (m)	Level of Service (LOS)
		East	0.86	21	96	В
	DM	North	0.87	28	117	В
	PIVI	West	0.76	19	83	В
Goodall Street/		All	0.87	23	117	В
Dunmore Street		East	0.64	15	64	В
	Cat	North	0.64	26	56	В
	Sat	West	0.62	17	90	В
		All	0.64	19	90	В
		South	0.77	22	71	В
	514	East	0.82	24	128	В
	PM	North	0.47	16	45	В
Pendle Way/		All	0.82	22	128	В
Dunmore Street	Sat	South	0.68	15	49	В
		East	0.81	25	86	В
		North	0.47	14	43	А
		All	0.81	18	86	В
	PM	South	0.50	63	37	E
		East	0.81	17	188	В
		North	0.87	66	74	E
		West	0.48	23	147	В
Great Western		All	0.87	23	188	В
Highway/ Jones Street		South	0.69	63	40	E
		East	0.66	22	116	В
	Sat	North	1.13	116	93	F
		West	0.46	20	122	В
		All	1.13	30	122	С
		South	0.79	39	301	С
		East	1.03	77	330	F
	PM	North	0.84	39	337	С
		West	0.70	60	107	E
Cumberland		All	1.03	46	337	D
Dunmore Street		South	0.72	33	192	С
		East	1.10	115	323	F
	Sat	North	0.76	32	192	С
		West	0.72	56	111	D
		All	1.10	45	323	D



Intersection	Peak	Leg	Degree of Saturation (DOS)	Average Delay (sec)	95 th Percentile (m)	Level of Service (LOS)
		South	0.68	12	178	А
		East	1.04	116	200	F
	PM	North	0.66	11	146	А
		West	1.11	125	95	F
Cumberland		All	1.11	23	200	В
Street		South	0.54	15	159	В
		East	0.65	60	55	E
	Sat	North	0.49	14	138	А
		West	1.14	161	119	F
		All	1.14	23	159	В
		South	0.93	50	58	D
	PM	North	1.61	611	438	F
		West	0.30	8	0	А
Pendle Way/		All	1.61	103	438	F
Gilba Road	Sat	South	1.16	205	190	F
		North	2.06	1015	632	F
		West	0.34	8	0	А
		All	2.06	201	632	F
		South	0.92	38	122	С
		East	0.97	33	113	С
	PM	West	0.65	21	139	В
Goodall Street/		All	0.97	30	139	С
Avenue	Sat	South	1.13	97	233	F
		East	0.73	31	102	С
		West	0.47	16	81	В
		All	1.13	47	233	D
		South	1.10	116	176	F
		East	1.00	36	233	С
	PM	North	0.93	71	253	F
		West	0.63	37	159	С
Cumberland		All	1.10	50	253	D
Highway/ Pendle Wav		South	0.95	73	130	F
		East	1.00	37	124	С
	Sat	North	0.85	57	140	E
		West	0.60	38	157	С
		All	1.00	45	157	D
13S1210200 Jones Street-Rogers Street Giveway / Yield (Two-Way)

Movem	Average Level of 95% Back of Overage Prop Effective Average											
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h	
South: J	lones Str	eet										
2	Т	344	2.0	0.196	1.7	LOS A	1.5	10.4	0.51	0.00	44.1	
3	R	21	2.0	0.196	8.5	LOS A	1.5	10.4	0.51	0.89	43.0	
Approac	ch	365	2.0	0.196	2.1	NA	1.5	10.4	0.51	0.05	44.0	
East: Ro	ogers Str	eet										
4	L	25	2.0	0.039	8.6	LOS A	0.1	1.0	0.45	0.63	41.4	
6	R	11	2.0	0.039	8.9	LOS A	0.1	1.0	0.45	0.78	41.3	
Approac	ch	36	2.0	0.039	8.7	LOS A	0.1	1.0	0.45	0.67	41.4	
North: J	ones Stre	eet										
7	L	9	2.0	0.197	6.5	LOS A	0.0	0.0	0.00	0.91	43.3	
8	Т	369	2.0	0.197	0.0	LOS A	0.0	0.0	0.00	0.00	50.0	
Approac	ch	379	2.0	0.197	0.2	NA	0.0	0.0	0.00	0.02	49.8	
All Vehic	cles	780	2.0	0.197	1.4	NA	1.5	10.4	0.26	0.07	46.5	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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13S1210200 Jones Street-Rogers Street Giveway / Yield (Two-Way)

Movem	ovement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h		
South: J	ones St	treet											
2	Т	260	2.0	0.152	1.3	LOS A	1.0	7.3	0.45	0.00	44.7		
3	R	22	2.0	0.152	8.1	LOS A	1.0	7.3	0.45	0.87	43.0		
Approac	h	282	2.0	0.152	1.8	NA	1.0	7.3	0.45	0.07	44.5		
East: Ro	ogers St	reet											
4	L	29	2.0	0.041	8.1	LOS A	0.2	1.1	0.40	0.61	41.8		
6	R	13	2.0	0.041	8.4	LOS A	0.2	1.1	0.40	0.74	41.7		
Approac	h	42	2.0	0.041	8.2	LOS A	0.2	1.1	0.40	0.65	41.8		
North: J	ones St	reet											
7	L	21	2.0	0.169	6.5	LOS A	0.0	0.0	0.00	0.89	43.3		
8	Т	303	2.0	0.169	0.0	LOS A	0.0	0.0	0.00	0.00	50.0		
Approac	h	324	2.0	0.169	0.4	NA	0.0	0.0	0.00	0.06	49.5		
All Vehic	cles	648	2.0	0.169	1.5	NA	1.0	7.3	0.22	0.10	46.7		

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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13S1210200 Jones Street-Oatlands Street Giveway / Yield (Two-Way)

Movem	Movement Performance - Vehicles											
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h	
South: J	lones St	reet										
2	Т	511	2.0	0.282	2.7	LOS A	2.6	18.3	0.64	0.00	42.8	
3	R	19	2.0	0.282	9.5	LOS A	2.6	18.3	0.64	0.94	42.6	
Approac	ch	529	2.0	0.282	2.9	NA	2.6	18.3	0.64	0.03	42.8	
East: Oa	atlands S	Street										
4	L	29	2.0	0.089	10.8	LOS A	0.3	2.2	0.56	0.71	39.6	
6	R	25	2.0	0.089	11.2	LOS A	0.3	2.2	0.56	0.86	39.5	
Approac	h	55	2.0	0.089	11.0	LOS A	0.3	2.2	0.56	0.78	39.5	
North: J	ones Str	reet										
7	L	25	2.0	0.254	6.5	LOS A	0.0	0.0	0.00	0.90	43.3	
8	Т	463	2.0	0.254	0.0	LOS A	0.0	0.0	0.00	0.00	50.0	
Approac	h	488	2.0	0.254	0.3	NA	0.0	0.0	0.00	0.05	49.6	
All Vehic	cles	1073	2.0	0.282	2.2	NA	2.6	18.3	0.34	0.08	45.5	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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13S1210200 Jones Street-Oatlands Street Giveway / Yield (Two-Way)

Movem	Average Level of 95% Back of Queue Prop Effective Average												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h		
South: J	lones St	treet											
2	Т	441	2.0	0.252	2.9	LOS A	2.3	16.2	0.64	0.00	42.8		
3	R	24	2.0	0.252	9.7	LOS A	2.3	16.2	0.64	0.94	42.4		
Approac	ch	465	2.0	0.252	3.3	NA	2.3	16.2	0.64	0.05	42.8		
East: Oa	atland S	treet											
4	L	17	2.0	0.078	11.4	LOS A	0.3	1.9	0.60	0.73	39.1		
6	R	26	2.0	0.078	11.8	LOS A	0.3	1.9	0.60	0.86	39.0		
Approac	ch	43	2.0	0.078	11.7	LOS A	0.3	1.9	0.60	0.81	39.0		
North: J	ones St	reet											
7	L	31	2.0	0.275	6.5	LOS A	0.0	0.0	0.00	0.90	43.3		
8	Т	497	2.0	0.275	0.0	LOS A	0.0	0.0	0.00	0.00	50.0		
Approac	h	527	2.0	0.275	0.4	NA	0.0	0.0	0.00	0.05	49.6		
All Vehic	cles	1036	2.0	0.275	2.1	NA	2.3	16.2	0.31	0.08	45.8		

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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13S1210200 Jones Street-Smith Street Roundabout

Movem	ient Per	formance ·	- Vehicles								
Mov ID	Turn	Demand Flow	HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
0 11 1		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: J	ones Str	eet									
1	L	35	2.0	0.525	11.6	LOS A	4.2	29.6	0.79	0.92	39.4
2	Т	303	2.0	0.525	10.6	LOS A	4.2	29.6	0.79	0.89	39.7
3	R	35	2.0	0.525	14.7	LOS B	4.2	29.6	0.79	0.97	37.9
Approac	:h	373	2.0	0.525	11.1	LOS A	4.2	29.6	0.79	0.90	39.5
East: Sr	nith Stree	et									
4	L	69	2.0	0.515	8.4	LOS A	3.8	27.3	0.65	0.72	41.5
5	Т	249	2.0	0.515	7.5	LOS A	3.8	27.3	0.65	0.67	41.5
6	R	151	2.0	0.515	11.5	LOS A	3.8	27.3	0.65	0.80	39.8
Approac	h	469	2.0	0.515	8.9	LOS A	3.8	27.3	0.65	0.72	40.9
North: J	ones Stre	eet									
7	L	184	2.0	0.420	7.3	LOS A	3.0	21.4	0.50	0.62	42.0
8	Т	178	2.0	0.420	6.4	LOS A	3.0	21.4	0.50	0.56	42.2
9	R	71	2.0	0.420	10.4	LOS A	3.0	21.4	0.50	0.74	40.5
Approac	h	433	2.0	0.420	7.4	LOS A	3.0	21.4	0.50	0.62	41.8
West: S	mith Stre	et									
10	L	72	2.0	0.291	9.8	LOS A	1.7	12.4	0.69	0.80	40.7
11	Т	118	2.0	0.291	8.8	LOS A	1.7	12.4	0.69	0.76	41.0
12	R	14	2.0	0.291	12.9	LOS A	1.7	12.4	0.69	0.87	38.9
Approac	h	203	2.0	0.291	9.4	LOS A	1.7	12.4	0.69	0.78	40.7
All Vehic	cles	1478	2.0	0.525	9.1	LOS A	4.2	29.6	0.65	0.74	40.8

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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13S1210200 Jones Street-Smith Street Roundabout

Moven	nent Per	formance	- Vehicles								
M 10	T	Demand	111.4	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
MOV ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
Coutby	lanaa Ctr	veh/h	%	V/C	Sec		veh	m		per veh	km/h
South: J	Jones Str	eel				1001		40.0		0.70	
1	L	24	2.0	0.364	8.6	LOSA	2.3	16.2	0.60	0.73	41.8
2	Т	241	2.0	0.364	7.6	LOS A	2.3	16.2	0.60	0.68	41.9
3	R	40	2.0	0.364	11.6	LOS A	2.3	16.2	0.60	0.83	39.8
Approa	ch	305	2.0	0.364	8.2	LOS A	2.3	16.2	0.60	0.70	41.6
East: Sr	mith Stree	et									
4	L	21	2.0	0.306	8.6	LOS A	1.8	13.1	0.60	0.72	41.4
5	Т	99	2.0	0.306	7.6	LOS A	1.8	13.1	0.60	0.67	41.6
6	R	129	2.0	0.306	11.7	LOS A	1.8	13.1	0.60	0.80	39.5
Approa	ch	249	2.0	0.306	9.8	LOS A	1.8	13.1	0.60	0.74	40.5
North: J	ones Stre	eet									
7	L	187	2.0	0.491	7.5	LOS A	3.8	26.7	0.54	0.64	41.9
8	Т	235	2.0	0.491	6.6	LOS A	3.8	26.7	0.54	0.58	42.0
9	R	84	2.0	0.491	10.6	LOS A	3.8	26.7	0.54	0.74	40.4
Approad	ch	506	2.0	0.491	7.6	LOS A	3.8	26.7	0.54	0.63	41.7
West: S	mith Stre	et									
10	L	74	2.0	0.274	9.1	LOS A	1.6	11.2	0.62	0.75	41.2
11	Т	118	2.0	0.274	8.1	LOS A	1.6	11.2	0.62	0.70	41.6
12	R	18	2.0	0.274	12.2	LOS A	1.6	11.2	0.62	0.84	39.4
Approad	ch	209	2.0	0.274	8.8	LOS A	1.6	11.2	0.62	0.73	41.3
All Vehi	cles	1271	2.0	0.491	8.4	LOS A	3.8	26.7	0.58	0.69	41.3

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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13S1210200 Jones Street-Dunmore Street Roundabout

Moven	nent Per	formance	- Vehicles								
	-	Demand	1.15.7	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
0 11		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Jones Str	eet									
1	L	237	2.0	0.790	28.0	LOS B	9.6	68.3	1.00	1.30	29.8
2	Т	62	2.0	0.790	27.1	LOS B	9.6	68.3	1.00	1.30	29.9
3	R	56	2.0	0.790	31.1	LOS C	9.6	68.3	1.00	1.30	29.1
Approa	ch	355	2.0	0.790	28.4	LOS B	9.6	68.3	1.00	1.30	29.7
East: D	unmore S	Street									
4	L	106	2.0	0.919	23.7	LOS B	23.2	165.4	1.00	1.30	32.1
5	Т	691	2.0	0.919	22.8	LOS B	23.2	165.4	1.00	1.30	32.2
6	R	26	2.0	0.919	26.8	LOS B	23.2	165.4	1.00	1.30	31.3
Approa	ch	823	2.0	0.919	23.0	LOS B	23.2	165.4	1.00	1.30	32.1
North: J	ones Str	eet									
7	L	14	2.0	0.146	10.4	LOS A	0.8	5.8	0.69	0.78	39.9
8	Т	21	2.0	0.146	9.4	LOS A	0.8	5.8	0.69	0.74	40.2
9	R	56	2.0	0.146	13.5	LOS A	0.8	5.8	0.69	0.83	38.2
Approa	ch	91	2.0	0.146	12.1	LOS A	0.8	5.8	0.69	0.80	38.9
West: D	unmore	Street									
10	L	66	2.0	0.567	7.3	LOS A	5.2	37.1	0.58	0.62	41.7
11	Т	312	2.0	0.567	6.4	LOS A	5.2	37.1	0.58	0.56	41.8
12	R	238	2.0	0.567	10.4	LOS A	5.2	37.1	0.58	0.71	40.4
Approa	ch	616	2.0	0.567	8.0	LOS A	5.2	37.1	0.58	0.63	41.2
All Vehi	cles	1884	2.0	0.919	18.6	LOS B	23.2	165.4	0.85	1.06	34.4

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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13S1210200 Jones Street-Dunmore Street Roundabout

Moven	nent Per	formance	- Vehicles								
		Demand		Deg.	Average	Level of	95% Back of	of Queue	Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
0, 11		veh/h	%	v/c	sec	_	veh	m	_	per veh	km/h
South:	Jones Str	eet									
1	L	176	2.0	0.394	10.5	LOS A	2.5	18.1	0.77	0.85	39.9
2	Т	23	2.0	0.394	9.5	LOS A	2.5	18.1	0.77	0.82	40.1
3	R	60	2.0	0.394	13.5	LOS A	2.5	18.1	0.77	0.89	38.2
Approa	ch	259	2.0	0.394	11.1	LOS A	2.5	18.1	0.77	0.85	39.5
East: D	unmore S	Street									
4	L	65	2.0	0.614	9.9	LOS A	5.7	40.3	0.73	0.81	40.7
5	Т	469	2.0	0.614	9.0	LOS A	5.7	40.3	0.73	0.77	41.0
6	R	18	2.0	0.614	13.0	LOS A	5.7	40.3	0.73	0.88	39.0
Approa	ch	553	2.0	0.614	9.2	LOS A	5.7	40.3	0.73	0.78	40.9
North: J	ones Str	eet									
7	L	19	2.0	0.144	11.1	LOS A	0.8	5.7	0.72	0.80	39.4
8	Т	25	2.0	0.144	10.2	LOS A	0.8	5.7	0.72	0.76	39.7
9	R	40	2.0	0.144	14.2	LOS A	0.8	5.7	0.72	0.86	37.8
Approa	ch	84	2.0	0.144	12.3	LOS A	0.8	5.7	0.72	0.82	38.7
West: D	unmore	Street									
10	L	40	2.0	0.556	6.9	LOS A	5.1	36.5	0.47	0.58	42.1
11	т	396	2.0	0.556	5.9	LOS A	5.1	36.5	0.47	0.51	42.3
12	R	222	2.0	0.556	9.9	LOS A	5.1	36.5	0.47	0.70	40.6
Approa	ch	658	2.0	0.556	7.3	LOS A	5.1	36.5	0.47	0.58	41.7
All Vehi	cles	1554	2.0	0.614	8.9	LOS A	5.7	40.3	0.63	0.71	40.9

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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Dunmore Street-Goodall Street

Signals - Fixed Time Cycle Time = 60 seconds (Optimum Cycle Time - Minimum Delay)

Movem	ent Pe	erformance - V	Vehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
East: Du	inmore	Street									
5	Т	608	2.0	0.709	12.5	LOS A	13.5	96.2	0.78	0.71	36.8
6	R	340	2.0	0.857	37.4	LOS C	10.2	72.8	1.00	1.13	26.3
Approac	h	948	2.0	0.857	21.4	LOS B	13.5	96.2	0.86	0.86	32.2
North: G	oodall :	Street									
7	L	335	2.0	0.585	15.1	LOS B	5.6	40.1	0.61	0.76	36.7
9	R	476	2.0	0.866	37.0	LOS C	16.5	117.4	1.00	1.03	26.4
Approac	:h	811	2.0	0.866	28.0	LOS B	16.5	117.4	0.84	0.92	29.9
West: D	unmore	Street									
10	L	277	2.0	0.269	10.7	LOS A	3.2	22.6	0.42	0.71	39.8
11	Т	391	2.0	0.761	25.0	LOS B	11.7	83.1	0.98	0.93	29.9
Approac	h	667	2.0	0.761	19.0	LOS B	11.7	83.1	0.75	0.84	33.3
All Vehic	les	2426	2.0	0.866	23.0	LOS B	16.5	117.4	0.82	0.87	31.7

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	nent Performance -	Pedestrians	3					
MoviD	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective
	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P5	Across N approach	53	24.3	LOS C	0.1	0.1	0.90	0.90
P7	Across W approach	53	22.5	LOS C	0.1	0.1	0.87	0.87
All Ped	estrians	106	23.4	LOS C			0.88	0.88

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Dunmore Street-Goodall Street

Signals - Fixed Time Cycle Time = 70 seconds (Optimum Cycle Time - Minimum Delay)

Movem	ent Pe	erformance - V	/ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
East: Du	inmore	Street									
5	Т	489	2.0	0.545	8.1	LOS A	8.9	63.7	0.58	0.51	40.4
6	R	301	2.0	0.643	27.1	LOS B	8.7	62.2	0.95	0.89	30.3
Approac	:h	791	2.0	0.643	15.3	LOS B	8.9	63.7	0.72	0.65	35.8
North: G	Goodall	Street									
7	L	294	2.0	0.638	20.1	LOS B	6.6	47.1	0.69	0.78	33.7
9	R	251	2.0	0.599	33.4	LOS C	7.9	56.4	0.95	0.82	27.7
Approac	h	544	2.0	0.638	26.2	LOS B	7.9	56.4	0.81	0.80	30.7
West: D	unmore	Street									
10	L	198	2.0	0.203	11.1	LOS A	2.9	20.6	0.47	0.73	39.5
11	Т	475	2.0	0.622	18.7	LOS B	12.6	89.7	0.85	0.73	33.0
Approac	h	673	2.0	0.622	16.5	LOS B	12.6	89.7	0.74	0.73	34.7
All Vehic	les	2007	2.0	0.643	18.6	LOS B	12.6	89.7	0.75	0.72	33.9

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Mover	nent Performance -	Pedestrians	S					
	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective
	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P5	Across N approach	53	20.8	LOS C	0.1	0.1	0.77	0.77
P7	Across W approach	53	29.3	LOS C	0.1	0.1	0.91	0.91
All Ped	estrians	106	25.0	LOS C			0.84	0.84

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Dunmore Street-Pendle Way

Signals - Fixed Time Cycle Time = 65 seconds (Optimum Cycle Time - Minimum Delay)

Movem	Movement Performance - Vehicles										
Mov ID	Turn	Demand Flow veh/ <u>h</u>	HV %	Deg. Satn v/c	Average Delay se <u>c</u>	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/ <u>h</u>
South: F	Pendle V	Vay									
2	Т	331	2.0	0.372	12.3	LOS A	6.9	48.9	0.69	0.59	37.1
3	R	329	2.0	0.773	32.1	LOS C	9.9	70.5	0.99	1.01	28.2
Approac	h	660	2.0	0.773	22.2	LOS B	9.9	70.5	0.84	0.80	32.0
East: Du	inmore	Street									
4	L	517	2.0	0.496	15.5	LOS B	9.9	70.4	0.64	0.79	36.4
6	R	534	2.0	0.824	32.8	LOS C	18.0	128.3	0.98	0.96	27.9
Approac	h	1051	2.0	0.824	24.3	LOS B	18.0	128.3	0.81	0.88	31.5
North: P	endle V	Vay									
7	L	325	2.0	0.315	10.5	LOS A	3.8	27.3	0.40	0.71	40.0
8	Т	224	2.0	0.473	23.2	LOS B	6.3	44.6	0.90	0.74	30.7
Approac	h	549	2.0	0.473	15.7	LOS B	6.3	44.6	0.60	0.72	35.6
All Vehic	les	2260	2.0	0.824	21.6	LOS B	18.0	128.3	0.77	0.82	32.6

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians												
MoviD	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective				
	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate				
		ped/h	sec		ped	m		per ped				
P1	Across S approach	53	11.1	LOS B	0.1	0.1	0.58	0.58				
P3	Across E approach	53	26.8	LOS C	0.1	0.1	0.91	0.91				
All Pede	estrians	106	18.9	LOS B			0.75	0.75				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Dunmore Street-Pendle Way

Signals - Fixed Time Cycle Time = 55 seconds (Optimum Cycle Time - Minimum Delay)

Movem	Movement Performance - Vehicles											
Mov ID	Turn	Demand Flow veh/ <u>h</u>	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles ve <u>h</u>	of Queue Distance m_	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h	
South: P	Pendle V	Way										
2	Т	362	2.0	0.369	8.9	LOS A	5.9	42.3	0.64	0.55	39.6	
3	R	289	2.0	0.675	23.2	LOS B	6.9	49.4	0.96	0.87	32.1	
Approac	h	652	2.0	0.675	15.2	LOS B	6.9	49.4	0.79	0.70	35.9	
East: Du	inmore	Street										
4	L	335	2.0	0.372	16.0	LOS B	5.7	40.4	0.66	0.77	36.1	
6	R	406	2.0	0.814	32.0	LOS C	12.0	85.6	0.99	0.98	28.2	
Approac	h	741	2.0	0.814	24.8	LOS B	12.0	85.6	0.84	0.88	31.3	
North: P	endle V	Vay										
7	L	393	2.0	0.365	10.5	LOS A	4.4	31.3	0.45	0.73	39.9	
8	Т	265	2.0	0.474	17.8	LOS B	6.0	43.1	0.87	0.73	33.5	
Approac	h	658	2.0	0.474	13.5	LOS A	6.0	43.1	0.62	0.73	37.0	
All Vehic	les	2051	2.0	0.814	18.1	LOS B	12.0	85.6	0.75	0.77	34.4	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians												
May ID	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective				
	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate				
		ped/h	sec		ped	m		per ped				
P1	Across S approach	53	13.1	LOS B	0.1	0.1	0.69	0.69				
P3	Across E approach	53	21.8	LOS C	0.1	0.1	0.89	0.89				
All Pede	estrians	106	17.5	LOS B			0.79	0.79				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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13S1210200 - Bonds Pendle Hill Additional Modelling Great Western Highway / Jones Street Post Development 2027 Thursday PM Peak Signals - Fixed Time Cycle Time = 140 seconds (User-Given Phase Times)

Movement Performance - Vehicles											
Mov ID	Turn	Demand	HV	Deg.	Average	Level of	95% Back (of Queue	Prop.	Effective Stop Rate	Average
		veh/h	%	V/C	sec		venicies	m	Queueu	ber veh	km/h
South: E	Berith Rd	- S									
4	L	51	2.1	0.499	55.4	LOS D	2.8	19.8	0.84	0.73	24.2
5	Т	21	0.0	0.405	61.3	LOS E	5.2	36.9	0.96	0.76	19.2
6	R	60	1.8	0.405	69.0	LOS E	5.2	36.9	0.96	0.78	21.5
Approac	:h	132	1.6	0.499	62.6	LOS E	5.2	36.9	0.91	0.76	22.1
East: Gt	Westerr	i Hwy - E									
7	L	71	0.0	0.574	23.0	LOS B	26.3	186.1	0.58	1.09	45.4
8	Т	2153	1.6	0.574	12.9	LOS A	26.5	188.1	0.58	0.53	53.1
9	R	203	0.0	0.806	55.0	LOS D	11.1	77.8	1.00	0.87	25.6
Approac	h	2426	1.4	0.806	16.7	LOS B	26.5	188.1	0.62	0.58	49.1
North: J	ones St -	N									
10	L	100	0.0	0.512	39.4	LOS C	4.5	31.6	0.71	0.75	28.8
11	Т	35	0.0	0.869	78.7	LOS F	10.5	74.2	1.00	1.00	16.5
12	R	103	1.0	0.869	86.4	LOS F	10.5	74.2	1.00	1.00	18.6
Approac	h	238	0.4	0.869	65.5	LOS E	10.5	74.2	0.88	0.89	21.5
West: G	t Wester	n Hwy - W									
1	L	86	0.0	0.478	31.0	LOS C	20.5	145.8	0.67	1.04	39.1
2	Т	1423	2.0	0.478	20.4	LOS B	20.7	147.3	0.66	0.59	45.3
3	R	51	2.1	0.483	83.0	LOS F	3.5	25.2	1.00	0.75	19.0
Approac	:h	1560	1.9	0.483	23.1	LOS B	20.7	147.3	0.67	0.62	43.4
All Vehic	cles	4356	1.5	0.869	23.1	LOS B	26.5	188.1	0.66	0.62	42.6

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped				
P3	Across S approach	53	11.2	LOS B	0.1	0.1	0.40	0.40				
P7	Across N approach	53	19.0	LOS B	0.1	0.1	0.52	0.52				
P1	Across W approach	53	60.4	LOS F	0.2	0.2	0.93	0.93				
All Ped	estrians	159	30.2	LOS D			0.62	0.62				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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13S1210200 - Bonds Pendle Hill Additional Modelling Great Western Highway / Jones Street Post Development 2027 Saturday Peak Signals - Fixed Time Cycle Time = 130 seconds (User-Given Phase Times)

Movement Performance - Vehicles											
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: E	Berith Rd	- S									
4	L	75	1.4	0.694	56.3	LOS D	4.2	29.5	0.87	0.83	21.2
5	Т	18	0.0	0.577	63.2	LOS E	5.6	39.7	1.00	0.79	21.1
6	R	69	3.0	0.577	69.3	LOS E	5.6	39.7	1.00	0.79	18.9
Approac	h	162	1.9	0.694	62.6	LOS E	5.6	39.7	0.94	0.81	20.2
East: Gt	Westerr	ı Hwy - E									
7	L	88	1.2	0.317	24.2	LOS B	14.7	105.5	0.65	1.04	44.7
8	Т	1119	3.1	0.317	17.2	LOS B	16.2	116.3	0.68	0.67	47.8
9	R	159	2.0	0.664	57.4	LOS E	8.0	56.8	1.00	0.86	24.9
Approac	h	1366	2.9	0.664	22.3	LOS B	16.2	116.3	0.71	0.72	43.1
North: J	ones St -	N									
10	L	140	0.0	0.706	45.2	LOS D	6.8	47.8	0.76	0.82	27.0
11	Т	35	0.0	1.130	202.4	LOS F	13.1	93.2	1.00	1.38	8.3
12	R	73	2.9	1.130	210.2	LOS F	13.1	93.2	1.00	1.38	9.6
Approac	h	247	0.9	1.130	115.7	LOS F	13.1	93.2	0.87	1.06	14.8
West: G	t Wester	n Hwy - W									
1	L	63	0.0	0.438	26.6	LOS B	16.9	119.9	0.61	1.06	42.3
2	Т	1392	1.8	0.438	16.4	LOS B	17.2	122.0	0.61	0.54	49.2
3	R	72	1.5	0.460	73.7	LOS F	4.5	32.0	0.99	0.77	20.8
Approac	h	1526	1.7	0.460	19.5	LOS B	17.2	122.0	0.63	0.58	46.4
All Vehic	cles	3302	2.1	1.130	30.0	LOS C	17.2	122.0	0.70	0.68	37.1

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped				
P3	Across S approach	53	10.8	LOS B	0.1	0.1	0.41	0.41				
P7	Across N approach	53	16.3	LOS B	0.1	0.1	0.50	0.50				
P1	Across W approach	53	58.2	LOS E	0.2	0.2	0.95	0.95				
All Ped	estrians	159	28.4	LOS C			0.62	0.62				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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13S1210200 - Bonds Pendle Hill Additional Modelling Cumberland Highway / Dunmore Street Post Development 2027 Thursday PM Peak Signals - Fixed Time Cycle Time = 140 seconds (User-Given Phase Times)

Movement Performance - Vehicles											
	-	Demand		Deg.	Average	Level of	95% Back o	of Queue	Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
0 11 0		veh/h	%	V/C	sec		veh	m		per veh	km/h
South: C	Sumberla	ind Hwy - S									
1	L	93	0.0	0.794	46.7	LOS D	41.8	298.3	0.90	0.94	28.9
2	Т	2180	2.6	0.794	38.2	LOS C	42.1	301.3	0.89	0.81	30.3
3	R	49	0.0	0.287	74.1	LOS F	3.2	22.7	0.97	0.75	19.9
Approac	h	2322	2.4	0.794	39.3	LOS C	42.1	301.3	0.89	0.82	29.9
East: Du	Inmore S	St - E									
4	L	74	0.0	0.771	42.3	LOS C	17.9	125.9	0.76	0.88	27.1
5	Т	528	1.0	1.028	64.5	LOS E	46.8	329.7	0.87	0.94	18.9
6	R	302	0.3	1.028	106.1	LOS F	46.8	329.7	1.00	1.24	15.5
Approac	h	904	0.7	1.028	76.6	LOS F	46.8	329.7	0.91	1.04	18.0
North: C	umberla	nd Hwy - N									
7	L	347	1.2	0.579	25.4	LOS B	8.9	62.8	0.49	0.77	37.3
8	Т	2403	3.9	0.840	39.5	LOS C	46.6	337.0	0.92	0.85	29.8
9	R	203	2.6	0.832	54.8	LOS D	10.1	72.4	1.00	0.93	24.4
Approac	h	2954	3.5	0.840	38.9	LOS C	46.6	337.0	0.88	0.85	30.1
West: D	unmore	St - W									
10	L	119	0.0	0.696	63.9	LOS E	15.2	107.0	0.97	0.85	21.6
11	Т	267	1.6	0.696	57.1	LOS E	15.2	107.0	0.98	0.84	20.1
12	R	63	3.3	0.696	65.3	LOS E	13.8	98.3	0.99	0.86	21.5
Approac	h	449	1.4	0.696	60.0	LOS E	15.2	107.0	0.98	0.84	20.7
All Vehic	cles	6629	2.6	1.028	45.6	LOS D	46.8	337.0	0.89	0.86	26.8

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped				
P1	Across S approach	53	39.4	LOS D	0.2	0.2	0.75	0.75				
P3	Across E approach	53	32.9	LOS D	0.1	0.1	0.69	0.69				
P5	Across N approach	53	58.5	LOS E	0.2	0.2	0.91	0.91				
P7	Across W approach	53	32.9	LOS D	0.1	0.1	0.69	0.69				
All Pedestrians		212	40.9	LOS E			0.76	0.76				



13S1210200 - Bonds Pendle Hill Additional Modelling Cumberland Highway / Dunmore Street Post Development 2027 Saturday Peak Signals - Fixed Time Cycle Time = 130 seconds (User-Given Phase Times)

Movement Performance - Vehicles											
Mov ID	Turn	Demand Flow veb/b	HV %	Deg. Satn	Average Delay	Level of Service	95% Back o Vehicles veh	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed km/b
South: C	Cumberla	and Hwy - S	/0	10			Ven				1111/11
1	L	58	0.0	0.722	38.9	LOS C	26.7	190.5	0.81	0.98	32.2
2	Т	1655	2.7	0.722	31.0	LOS C	26.8	192.2	0.80	0.74	33.8
3	R	81	1.3	0.636	76.1	LOS F	5.3	37.7	1.00	0.80	19.5
Approac	h	1794	2.5	0.722	33.3	LOS C	26.8	192.2	0.81	0.75	32.7
East: Du	unmore S	St - E									
4	L	63	3.3	0.657	34.4	LOS C	8.5	60.5	0.70	0.82	29.9
5	Т	289	0.4	1.095	88.1	LOS F	45.9	323.1	0.85	1.01	15.5
6	R	313	0.7	1.095	156.3	LOS F	45.9	323.1	1.00	1.44	11.6
Approac	h	665	0.8	1.095	115.0	LOS F	45.9	323.1	0.91	1.20	13.9
North: C	umberla	and Hwy - N									
7	L	353	1.5	0.752	32.6	LOS C	13.6	96.5	0.70	0.82	33.0
8	Т	1725	2.4	0.721	30.8	LOS C	26.8	191.7	0.80	0.74	34.0
9	R	161	0.0	0.758	41.4	LOS C	6.2	43.6	1.00	0.88	28.9
Approac	h	2239	2.1	0.758	31.9	LOS C	26.8	191.7	0.80	0.76	33.4
West: D	unmore	St - W									
10	L	141	0.0	0.719	58.7	LOS E	15.9	111.2	0.97	0.86	22.6
11	Т	243	0.4	0.719	52.2	LOS D	15.9	111.2	0.98	0.85	21.1
12	R	116	0.9	0.719	60.7	LOS E	14.3	100.4	0.99	0.88	22.3
Approac	h	500	0.4	0.719	56.0	LOS D	15.9	111.2	0.98	0.86	21.8
All Vehic	cles	5198	1.9	1.095	45.3	LOS D	45.9	323.1	0.83	0.82	26.9

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped				
P1	Across S approach	53	34.7	LOS D	0.1	0.1	0.73	0.73				
P3	Across E approach	53	28.4	LOS C	0.1	0.1	0.66	0.66				
P5	Across N approach	53	53.6	LOS E	0.2	0.2	0.91	0.91				
P7	Across W approach	53	28.4	LOS C	0.1	0.1	0.66	0.66				
All Pedestrians		212	36.3	LOS D			0.74	0.74				



13S1210200 - Bonds Pendle Hill Additional Modelling Cumberland Highway / Smith Street Post Development 2027 Thursday PM Peak Signals - Fixed Time Cycle Time = 140 seconds (User-Given Phase Times)

Movem	ent Pe	rformance - \	Vehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: C	Cumberla	and Hwy - S									
1	L	176	0.6	0.690	19.8	LOS B	24.9	177.5	0.50	0.94	40.0
2	Т	2301	2.6	0.690	9.7	LOS A	24.9	177.5	0.44	0.41	45.7
3	R	64	0.0	0.440	77.0	LOS F	4.4	30.5	0.99	0.76	19.2
Approac	h	2541	2.4	0.690	12.1	LOS A	24.9	177.5	0.46	0.45	43.7
East: Sr	nith St -	E									
4	L	67	0.0	0.521	63.9	LOS E	6.8	47.9	0.93	0.79	22.1
5	Т	280	1.5	1.042	124.5	LOS F	28.2	199.8	0.99	1.22	13.2
6	R	34	0.0	1.042	145.4	LOS F	28.2	199.8	1.00	1.31	12.3
Approac	h	381	1.1	1.042	115.7	LOS F	28.2	199.8	0.98	1.15	14.1
North: C	umberla	and Hwy - N									
7	L	49	2.1	0.664	17.6	LOS B	20.2	145.9	0.42	1.00	41.8
8	Т	2327	3.9	0.664	8.9	LOS A	20.2	145.9	0.40	0.37	46.6
9	R	56	1.9	0.387	76.7	LOS F	3.8	26.8	0.99	0.75	19.3
Approac	:h	2433	3.8	0.664	10.7	LOS A	20.2	145.9	0.42	0.39	45.1
West: S	mith St -	W									
10	L	36	0.0	0.555	65.1	LOS E	7.2	51.3	0.94	0.80	22.1
11	Т	131	4.0	1.110	108.3	LOS F	13.4	95.0	0.96	0.94	14.6
12	R	60	0.0	1.110	198.6	LOS F	13.4	95.0	1.00	1.26	9.4
Approac	:h	226	2.3	1.110	125.4	LOS F	13.4	95.0	0.97	1.01	13.3
All Vehic	cles	5581	2.9	1.110	23.2	LOS B	28.2	199.8	0.50	0.50	35.8

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped				
P1	Across S approach	53	59.4	LOS E	0.2	0.2	0.92	0.92				
P3	Across E approach	53	13.7	LOS B	0.1	0.1	0.44	0.44				
P5	Across N approach	53	59.4	LOS E	0.2	0.2	0.92	0.92				
P7	Across W approach	53	13.7	LOS B	0.1	0.1	0.44	0.44				
All Ped	estrians	212	36.6	LOS D			0.68	0.68				



13S1210200 - Bonds Pendle Hill Additional Modelling Cumberland Highway / Smith Street Post Development 2027 Saturday Peak Signals - Fixed Time Cycle Time = 130 seconds (User-Given Phase Times)

Movem	ent Pe	rformance - '	Vehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back (Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: C	Cumberla	and Hwy - S	,,,								
1	L	174	0.6	0.537	20.4	LOS B	22.0	156.7	0.57	0.92	39.7
2	Т	1866	2.4	0.537	12.2	LOS A	22.2	158.9	0.57	0.52	43.0
3	R	64	3.3	0.460	72.9	LOS F	4.1	29.4	1.00	0.76	20.0
Approac	h	2104	2.3	0.537	14.7	LOS B	22.2	158.9	0.58	0.56	41.3
East: Sn	nith St -	E									
4	L	46	2.3	0.186	48.8	LOS D	2.3	16.1	0.81	0.74	25.6
5	Т	101	0.0	0.651	62.7	LOS E	7.9	55.3	1.00	0.82	21.1
6	R	23	0.0	0.651	70.7	LOS F	7.9	55.3	1.00	0.82	21.1
Approac	:h	171	0.6	0.651	60.0	LOS E	7.9	55.3	0.95	0.80	22.2
North: C	umberla	and Hwy - N									
7	L	48	0.0	0.490	19.8	LOS B	19.3	137.6	0.54	0.97	40.4
8	Т	1815	2.6	0.490	11.6	LOS A	19.3	138.1	0.54	0.49	43.7
9	R	64	0.0	0.449	72.7	LOS F	4.1	28.5	1.00	0.76	20.0
Approac	h	1927	2.4	0.490	13.8	LOS A	19.3	138.1	0.56	0.51	41.9
West: Si	mith St -	W									
10	L	28	0.0	0.325	63.2	LOS E	4.2	29.2	0.94	0.78	22.4
11	Т	95	0.0	1.139	139.4	LOS F	16.9	119.1	0.97	1.08	12.0
12	R	83	1.3	1.139	219.1	LOS F	16.9	119.1	1.00	1.37	8.6
Approac	:h	206	0.5	1.139	161.0	LOS F	16.9	119.1	0.98	1.15	11.0
All Vehic	cles	4408	2.2	1.139	22.9	LOS B	22.2	158.9	0.60	0.58	35.7

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped					
P1	Across S approach	53	59.1	LOS E	0.2	0.2	0.95	0.95					
P3	Across E approach	53	11.2	LOS B	0.1	0.1	0.42	0.42					
P5	Across N approach	53	59.1	LOS E	0.2	0.2	0.95	0.95					
P7	Across W approach	53	11.2	LOS B	0.1	0.1	0.42	0.42					
All Pedestrians		212	35.2	LOS D			0.68	0.68					



13S1210200 - Bonds Pendle Hill Additional Modelling Gilba Road / Pendle Way Post Development 2027 Thursday PM Peak Giveway / Yield (Two-Way)

Movem	Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h		
South: P	endle V	Vay - S											
1	L	571	0.7	0.309	8.2	LOS A	0.0	0.0	0.00	0.67	49.0		
2	Т	226	0.5	0.932	50.2	LOS D	8.3	58.1	0.74	1.69	25.1		
Approac	h	797	0.7	0.932	20.1	LOS B	8.3	58.1	0.21	0.96	38.6		
North: P	endle W	/ay - N											
8	Т	173	3.7	1.606	610.9	LOS F	60.9	437.5	1.00	6.34	3.4		
9	R	60	1.8	1.606	612.2	LOS F	60.9	437.5	1.00	4.69	3.3		
Approac	h	233	3.2	1.606	611.2	LOS F	60.9	437.5	1.00	5.91	3.4		
West: Gi	lba Rd	- W											
10	L	193	0.5	0.299	8.2	LOS A	0.0	0.0	0.00	0.66	49.0		
12	R	360	0.9	0.299	8.3	LOS A	0.0	0.0	0.00	0.67	48.9		
Approac	h	553	0.8	0.299	8.2	NA	0.0	0.0	0.00	0.67	48.9		
All Vehic	les	1582	1.1	1.606	102.9	NA	60.9	437.5	0.25	1.59	15.6		

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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13S1210200 - Bonds Pendle Hill Additional Modelling Gilba Road / Pendle Way Post Development 2027 Saturday Peak Giveway / Yield (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h	
South: P	endle V	Vay - S										
1	L	454	1.4	0.247	8.2	LOS A	0.0	0.0	0.00	0.67	49.0	
2	Т	231	0.0	1.163	204.8	LOS F	27.1	189.5	1.00	3.42	9.0	
Approac	h	684	0.9	1.163	74.5	LOS F	27.1	189.5	0.34	1.59	19.6	
North: P	endle W	/ay - N										
8	Т	172	3.1	2.058	1013.6	LOS F	88.4	632.2	1.00	6.95	2.1	
9	R	85	1.2	2.058	1014.9	LOS F	88.4	632.2	1.00	5.62	2.1	
Approac	h	257	2.5	2.058	1014.0	LOS F	88.4	632.2	1.00	6.51	2.1	
West: Gi	ilba Rd ·	- W										
10	L	201	1.0	0.341	8.2	LOS A	0.0	0.0	0.00	0.66	49.0	
12	R	429	0.2	0.341	8.2	LOS A	0.0	0.0	0.00	0.67	48.9	
Approac	h	631	0.5	0.341	8.2	NA	0.0	0.0	0.00	0.67	48.9	
All Vehic	les	1572	1.0	2.058	201.4	NA	88.4	632.2	0.31	2.03	9.1	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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13S1210200 - Bonds Pendle Hill Additional Modelling Goodall Street / Wentworth Avenue Post Development 2027 Thursday PM Peak Signals - Fixed Time Cycle Time = 90 seconds (User-Given Phase Times)

Movement Performance - Vehicles											
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: G	Goodall	St - S									
1	L	360	1.5	0.610	15.7	LOS B	7.1	50.5	0.52	0.75	37.4
3	R	320	0.7	0.917	62.0	LOS E	17.4	122.3	1.00	1.06	20.8
Approac	h	680	1.1	0.917	37.5	LOS C	17.4	122.3	0.75	0.90	27.2
East: We	entworth	n Ave - E									
4	L	412	0.3	0.971	31.4	LOS C	11.6	81.6	0.97	0.86	31.1
5	Т	382	2.5	0.711	35.2	LOS C	15.8	113.2	0.98	0.85	29.1
Approac	h	794	1.3	0.971	33.2	LOS C	15.8	113.2	0.97	0.85	30.1
West: W	/entwort	h Ave - W									
11	Т	395	2.4	0.540	6.4	LOS A	6.6	47.1	0.45	0.39	49.1
12	R	539	2.0	0.649	32.4	LOS C	19.5	138.9	0.88	0.85	30.7
Approac	h	934	2.1	0.649	21.4	LOS B	19.5	138.9	0.70	0.66	36.7
All Vehic	cles	2407	1.6	0.971	29.9	LOS C	19.5	138.9	0.80	0.79	31.3

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians												
		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective				
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate				
		ped/h	sec		ped	m		per ped				
P7	Across W approach	53	36.5	LOS D	0.1	0.1	0.90	0.90				
All Ped	estrians	53	36.5	LOS D			0.90	0.90				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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13S1210200 - Bonds Pendle Hill Additional Modelling Goodall Street / Wentworth Avenue Post Development 2027 Saturday Peak Signals - Fixed Time Cycle Time = 90 seconds (User-Given Phase Times)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h	
South: G	Goodall	St - S										
1	L	357	1.5	0.605	16.6	LOS B	7.0	49.9	0.51	0.77	41.2	
3	R	324	0.6	1.127	184.7	LOS F	33.0	232.5	1.00	1.50	9.8	
Approac	h	681	1.1	1.127	96.6	LOS F	33.0	232.5	0.75	1.12	16.4	
East: We	entworth	h Ave - E										
4	L	297	0.4	0.726	28.0	LOS B	8.8	61.5	0.69	0.81	33.9	
5	Т	358	2.6	0.615	33.6	LOS C	14.2	101.7	0.95	0.80	29.8	
Approac	h	655	1.6	0.726	31.0	LOS C	14.2	101.7	0.83	0.81	31.5	
West: W	/entwort	th Ave - W										
11	Т	379	2.5	0.468	4.8	LOS A	5.5	39.5	0.38	0.32	51.3	
12	R	371	2.8	0.381	28.1	LOS B	11.3	80.8	0.74	0.81	33.9	
Approac	h	749	2.7	0.468	16.3	LOS B	11.3	80.8	0.56	0.56	40.9	
All Vehic	les	2085	1.8	1.127	47.2	LOS D	33.0	232.5	0.71	0.82	25.8	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians												
Mov ID	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective Stop Pate				
	Decomption	ped/h	sec	Service	pedestinan	Distance	Queueu	ber ped				
P7	Across W approach	53	39.2	LOS D	0.1	0.1	0.93	0.93				
All Pede	estrians	53	39.2	LOS D			0.93	0.93				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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13S1210200 - Bonds Pendle Hill Additional Modelling Great Western Highway / Pendle Way Post Development 2027 Thursday PM Peak Signals - Fixed Time Cycle Time = 140 seconds (User-Given Phase Times)

Movem	ent Pei	rformance - V	/ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: E	ttalong	Rd - S									
4	L	124	0.8	0.606	53.5	LOS D	6.7	47.5	0.84	0.77	24.2
5	Т	244	0.9	1.099	111.6	LOS F	25.1	176.2	1.00	1.05	14.3
6	R	123	0.0	1.099	187.0	LOS F	25.1	176.2	1.00	1.36	9.9
Approac	h	492	0.6	1.099	115.8	LOS F	25.1	176.2	0.96	1.06	14.2
East: Gt	Western	n Hwy - E									
7	L	261	0.4	0.695	41.9	LOS C	32.3	228.9	0.82	0.95	28.6
8	Т	1748	2.0	0.695	31.3	LOS C	32.7	232.6	0.79	0.72	30.8
<mark>9</mark>	R	<mark>156</mark>	1.3	<mark>1.000</mark> °	73.0	LOS F	10.4	73.4	1.00	0.80	20.0
Approac	h	2165	1.8	1.000	35.6	LOS C	32.7	232.6	0.81	0.75	29.4
North: P	endle W	/ay - N									
10	L	91	1.2	0.745	60.1	LOS E	17.6	124.8	0.96	0.89	23.3
11	Т	316	1.3	0.931	60.5	LOS E	35.3	252.5	0.98	0.91	21.3
12	R	333	3.2	0.931	84.3	LOS F	35.3	252.5	1.00	1.02	18.3
Approac	h	739	2.1	0.931	71.2	LOS F	35.3	252.5	0.99	0.96	20.0
West: G	t Wester	n Hwy - W									
1	L	144	5.8	0.544	40.1	LOS C	21.3	153.3	0.80	0.89	29.4
2	Т	1159	2.4	0.544	32.1	LOS C	22.3	158.9	0.80	0.71	30.5
3	R	134	0.8	0.633	73.8	LOS F	9.0	63.4	1.00	0.81	19.9
Approac	h	1437	2.6	0.633	36.8	LOS C	22.3	158.9	0.82	0.74	28.9
All Vehic	les	4833	1.9	1.099	49.5	LOS D	35.3	252.5	0.85	0.81	24.8

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

3 x = 1.00 due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

Movement Performance - Pedestrians													
	-	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective					
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate					
		ped/h	sec		ped	m		per ped					
P3	Across S approach	53	31.6	LOS D	0.1	0.1	0.67	0.67					
P5	Across E approach	53	59.4	LOS E	0.2	0.2	0.92	0.92					
P7	Across N approach	53	29.6	LOS C	0.1	0.1	0.65	0.65					
All Pede	estrians	159	40.2	LOS E			0.75	0.75					



13S1210200 - Bonds Pendle Hill Additional Modelling Great Western Highway / Pendle Way Post Development 2027 Saturday Peak Signals - Fixed Time Cycle Time = 130 seconds (User-Given Phase Times)

Movem	ent Per	rformance - V	/ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: E	ttalong l	Rd - S									
4	L	77	1.4	0.328	42.1	LOS C	3.4	24.4	0.75	0.75	27.8
5	Т	195	0.5	0.952	67.7	LOS E	18.4	129.8	0.98	0.89	20.1
6	R	163	1.3	0.952	93.5	LOS F	18.4	129.8	1.00	1.11	17.0
Approac	h	435	1.0	0.952	72.8	LOS F	18.4	129.8	0.95	0.95	19.7
East: Gt	Westerr	n Hwy - E									
7	L	171	0.6	0.460	38.1	LOS C	15.0	106.5	0.81	0.90	29.9
8	Т	908	3.5	0.460	31.9	LOS C	17.2	123.7	0.81	0.71	30.5
<mark>9</mark>	R	<mark>168</mark>	2.7	<mark>1.000</mark> °	66.0	LOS E	10.3	73.4	0.99	0.80	21.3
Approach 12		1247	3.0	1.000	37.4	LOS C	17.2	123.7	0.83	0.75	28.7
North: P	endle W	ay - N									
10	L	137	3.1	0.676	44.8	LOS D	10.1	72.0	0.98	0.84	27.4
11	Т	194	0.5	0.845	51.8	LOS D	19.8	139.9	0.99	0.89	23.1
12	R	184	1.7	0.845	71.2	LOS F	19.8	139.9	1.00	0.95	20.6
Approac	h	515	1.6	0.845	56.9	LOS E	19.8	139.9	0.99	0.90	23.1
West: G	t Wester	n Hwy - W									
1	L	139	1.5	0.600	43.1	LOS D	21.4	151.8	0.86	0.88	28.3
2	Т	1154	1.9	0.600	34.8	LOS C	22.1	157.0	0.86	0.76	29.3
3	R	105	1.0	0.481	64.2	LOS E	6.2	44.0	0.96	0.78	21.7
Approac	h	1398	1.8	0.600	37.9	LOS C	22.1	157.0	0.87	0.77	28.4
All Vehic	les	3595	2.1	1.000	44.6	LOS D	22.1	157.0	0.88	0.80	26.2

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

3 x = 1.00 due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

Movement Performance - Pedestrians													
		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective					
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate					
		ped/h	sec		ped	m		per ped					
P3	Across S approach	53	34.0	LOS D	0.1	0.1	0.72	0.72					
P5	Across E approach	53	59.1	LOS E	0.2	0.2	0.95	0.95					
P7	Across N approach	53	31.9	LOS D	0.1	0.1	0.70	0.70					
All Pede	estrians	159	41.7	LOS E			0.79	0.79					



Dunmore Street - Access Roundabout

Movem	Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h		
South: A	ccess												
1	L	102	2.0	0.443	18.7	LOS B	3.4	24.1	0.98	1.05	35.4		
3	R	54	2.0	0.443	22.1	LOS B	3.4	24.1	0.98	1.05	34.3		
Approac	h	156	2.0	0.443	19.9	LOS B	3.4	24.1	0.98	1.05	35.0		
East: Du	Inmore	Street											
4	L	93	2.0	0.862	13.4	LOS A	18.5	131.7	1.00	0.76	39.4		
5	Т	845	2.0	0.862	12.6	LOS A	18.5	131.7	1.00	0.76	39.5		
Approac	h	938	2.0	0.862	12.6	LOS A	18.5	131.7	1.00	0.76	39.5		
West: D	unmore	e Street											
11	Т	561	2.0	0.553	6.3	LOS A	6.4	45.8	0.41	0.48	43.3		
12	R	158	2.0	0.553	10.5	LOS A	6.4	45.8	0.41	0.69	41.4		
Approac	:h	719	2.0	0.553	7.3	LOS A	6.4	45.8	0.41	0.53	42.8		
All Vehic	les	1813	2.0	0.862	11.1	LOS A	18.5	131.7	0.76	0.69	40.3		

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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Dunmore Street - Access Roundabout

Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h	
South: A	ccess									·		
1	L	215	2.0	0.590	16.9	LOS B	5.6	39.7	0.94	1.08	36.5	
3	R	113	2.0	0.590	20.3	LOS B	5.6	39.7	0.94	1.09	35.2	
Approac	h	327	2.0	0.590	18.1	LOS B	5.6	39.7	0.94	1.08	36.0	
East: Du	inmore	Street										
4	L	113	2.0	0.720	11.5	LOS A	9.7	68.8	0.87	0.79	40.7	
5	Т	576	2.0	0.720	10.7	LOS A	9.7	68.8	0.87	0.77	40.9	
Approac	h	688	2.0	0.720	10.8	LOS A	9.7	68.8	0.87	0.78	40.9	
West: Du	unmore	e Street										
11	Т	546	2.0	0.667	7.4	LOS A	8.4	59.9	0.68	0.56	42.0	
12	R	215	2.0	0.667	11.5	LOS A	8.4	59.9	0.68	0.67	41.0	
Approac	h	761	2.0	0.667	8.5	LOS A	8.4	59.9	0.68	0.59	41.7	
All Vehic	les	1777	2.0	0.720	11.2	LOS A	9.7	68.8	0.80	0.75	40.2	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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John Street - Access Stop (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back (Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h	
South: J	ones S	treet										
4	L	223	2.0	0.301	7.3	LOS A	0.0	0.0	0.00	0.77	43.8	
5	Т	344	2.0	0.301	0.0	LOS A	0.0	0.0	0.00	0.00	51.3	
Approac	h	567	2.0	0.301	2.9	NA	0.0	0.0	0.00	0.30	48.1	
North: Jo	ones St	treet										
11	Т	374	2.0	0.240	3.2	LOS A	2.1	15.0	0.64	0.00	42.7	
12	R	45	2.0	0.240	10.6	LOS A	2.1	15.0	0.64	0.92	42.5	
Approac	h	419	2.0	0.240	4.0	NA	2.1	15.0	0.64	0.10	42.7	
West: Ac	ccess											
1	L	22	2.0	0.025	12.3	LOS A	0.1	0.7	0.47	0.87	40.5	
3	R	114	2.0	0.281	18.8	LOS B	1.1	7.7	0.73	1.04	36.3	
Approac	h	136	2.0	0.281	17.8	LOS B	1.1	7.7	0.69	1.01	36.9	
All Vehic	les	1122	2.0	0.301	5.1	NA	2.1	15.0	0.32	0.31	44.3	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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John Street - Access Stop (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h	
South: J	lones Str	eet										
4	L	241	2.0	0.254	7.3	LOS A	0.0	0.0	0.00	0.74	43.8	
5	Т	236	2.0	0.254	0.0	LOS A	0.0	0.0	0.00	0.00	51.3	
Approac	h	477	2.0	0.254	3.7	NA	0.0	0.0	0.00	0.38	47.2	
North: J	ones Stre	eet										
11	Т	286	2.0	0.190	2.3	LOS A	1.4	10.3	0.56	0.00	43.6	
12	R	46	2.0	0.190	9.7	LOS A	1.4	10.3	0.56	0.86	43.0	
Approac	h	333	2.0	0.190	3.3	NA	1.4	10.3	0.56	0.12	43.5	
West: A	ccess											
1	L	46	2.0	0.047	11.7	LOS A	0.2	1.3	0.42	0.88	40.9	
3	R	241	2.0	0.453	17.7	LOS B	2.3	16.7	0.70	1.11	36.9	
Approac	ch	287	2.0	0.453	16.7	LOS B	2.3	16.7	0.65	1.07	37.5	
All Vehic	cles	1097	2.0	0.453	7.0	NA	2.3	16.7	0.34	0.48	43.2	

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Attachment 4

Access Road Concept Layout

- Dunmore Street Roundabout
- Jones Street T- Intersection



